Isture of fee for subdivision- may are more after John Review Office Use Only Fee Paid Date Approved Permit Number Public Hearing: YES NO Agent Approval: YES NO TOWN OF EAST HAMPTON INLAND WETLANDS & WATERCOURSES AGENC Date: 1. Name of Applicant* Lake was Stufes Email: Whendak Phone Numbers: Home **Business** 860-267-4623, Cell <u>860</u> -Home Address: Street State/Zip Town Business Address: Street 244 Hiddle town ME Jown E Hampto State/Zip C7 * All applications MUST list contact phone numbers. If the applicant is a Limited Liability Corporation or a Corporation, provide the managing member's or responsible corporate officer's name, address, and telephone number. 2. Name of Property Owner (if different from Applicant): Phone Address: Street Town State/Zip As the legal owner of the property listed on this application I hereby consent to the proposed activities. I hereby authorize the members and agents of the Agency to inspect the subject land, at reasonable times, during the pendency of the application and for the life of the permit. Printed Name: Wayne Kand, Signature: // // Date: 3. Provide the applicant's interest in the land. 4. Site Location and Description: Assessor's Map 03A, Block Address: Street Note: It is the applicant's responsibility to provide the correct site address, map, block, and lot number for the legal notice. Provide a description of the land in sufficient detail to allow identification of the inland wetlands and watercourses, the area(s) (in acres or square feet) of wetlands or watercourses to be disturbed, soil type(s). and wetland vegetation. Area of Wetland to be disturbed: acres or sq. ft. Area of Watercourse to be disturbed _acres or sq. ft. 2_60` Area of Upland Review Area to be disturbed: acres or sq. ft.(Area within 100' of wetland) **TOTAL AREA OF DISTURBANCE** acres or sq. ft. Will fill be needed on site? Yes No If yes, how much fill is needed?_____ cubic yards The property contains (circle one or more) WETLANDS, BROOK, RIVER, INTERMITTANT STREAM, VERNAL POOL, SWAMP, OTHER Description of soil types site: Description of wetland_ vegetation: Name of Soil Scientist and date of survey: 5. Attach a written narrative of the purpose and description of the proposed activity and proposed erosion and sedimentation controls, best management practices, and mitigation measures which may be considered as a condition of issuing a permit for the proposed regulated activity including but not limited to; measures to: (1) prevent or minimize pollution or other environmental damage, (2) maintain or enhance existing environmental quality, or (3) in the following order of priority: restore, enhance or create productive wetland or watercourse resources. Depending on the complexity of the project, include the following: sequence of

(1) prevent or minimize pollution or other environmental damage, (2) maintain or enhance existing environmental quality, or (3) in the following order of priority: restore, enhance or create productive wetland or watercourse resources. Depending on the complexity of the project, include the following: sequence of operations, drainage computations with pre and post construction runoff quantities and runoff rates, plans clearly showing the drainage areas corresponding to the drainage computations, existing wetland inventory and functional assessment, soils report, construction plans signed by a certified soils scientist, licensed surveyor, and licensed professional engineer. Include a construction schedule, impacts to vegetation, and pictures that clearly show the existing conditions of all areas to be disturbed and/or cleared of vegetation.

6. Provide information of all alternatives considered. List all alternatives which would cause less or no environmental impact to wetlands or watercourses and state why the alternative as set forth in the application

was chosen. All such alternatives shall be diagramed on a site plan or drawing.



MBL:

Project# Address:

INLAND WETLANDS & WATERCOURSES AGENCY TOWN OF EAST HAMPTON

Minimum Requirements for Submission of Application to **Inland Wetlands and Watercourses Agency**

This form must be submitted with your application

Please check all that are being submitted:
Completed Application Form (4 Pages) Fee Paid Site Plan (Showing project location, extent of wetlands, dimensions, etc) – PDF & 4 Copies of 11 x 17s PDF & 4 CopiesProject Narrative – PDF & 4 Copies of 11 x 17s Soils Report (As Required) Stormwater Report (As Required) Completed Application Checklist (Page 3 of Application) Schedule a Site Visit with Planning & Zoning Official at time of Application
Date of Site Visit:

I certify that this application is complete:	11/2 /200
Signature of Applicant:	Date: 4/16/64

The Agency reserves the right to add additional requirements in accordance with the Regulations.

Only Complete Application Packages Will Be Accepted

03A-44-23A AQUARION WATER CO OF 600 LINDLEY ST BRIDGEPORT, CT 06606

03A-44-C-94 BB + G HOLDINGS LLC 10 PINE ST PLAINVILLE, CT 06062

18-44-23G-4 DOBLE DEEPAK & LANDANI 57 HIGHLAND TER EAST HAMPTON, CT 06424

03A-44-C-102 GOKEY JASON A 27 LAKEWOOD RD EAST HAMPTON, CT 06424

18-44-23G-6 IANNONE MATTHEW 59 HIGHLAND TER EAST HAMPTON, CT 06424

03A-44-C-101 NANE CORRADO + 29 LAKEWOOD DR EAST HAMPTON, CT 06424

18-44-23GRD-OS1 SKYLINE ESTATES LLC 244 MIDDLETOWN AVE EAST HAMPTON, CT 06424

03A-44-C-104 STRONG TIMOTHY A PO BOX 353 MARLBOROUGH, CT 06447

03A-44-C-109 TRAINO ANNA 15 LAKEWOOD RD EAST HAMPTON, CT 06424 03A-44-C-110 BARTHELL TAYLOR F 13 LAKEWOOD RD EAST HAMPTON, CT 06424

03A-44-C-97 BURKE MICHAEL M 35 LAKEWOOD RD EAST HAMPTON, CT 06424

03A-44-C-95 ENGELBERT KEITH 39 LAKEWOOD RD EAST HAMPTON, CT 06424

18-44-23G-1A HEBERT MATTHEW 53 HIGHLAND TER EAST HAMPTON, CT 06424

03A-44-C-99 JACKSON JACQUELINE PO BX 312 EAST HAMPTON, CT 06424

03A-44-C-107 PERKINS ROSALIE + 74 DIVIDEND RD ROCKY HILL, CT 06067

18-44-23G-2 SKYLINE ESTATES LLC 244 MIDDLETOWN AVE EAST HAMPTON, CT 06424

03A-44-C-114 SZYMASZEK JOHN J + NANCY 5 LAKEWOOD RD EAST HAMPTON, CT 06424

03A-44-C-98 US BANK TRUST NA TR/F 3701 REGENT BLVD SUITE 200 IRVING, TX 75063 03A-44-23 BB + G HOLDINGS LLC 10 PINE ST PLAINVILLE, CT 06062

03A-44-C-96 CATALINA PAMELA MAE + 37 LAKEWOOD RD EAST HAMPTON, CT 06424

24-44-18 ENGSTROM ALFRED J + SUN 11170 CHAMBERS COURT WOODSTOCK, MD 21163

03A-44-C-103 HOULE CYNTHIA 25 LAKEWOOD RD EAST HAMPTON, CT 06424

03A-44-C-112 MURPHY RICHARD M 9 LAKEWOOD RD EAST HAMPTON, CT 06424

18-44-23G-1 ROMAN MIGUEL A 51 HIGHLAND TER EAST HAMPTON, CT 06424

03A-44-25-1 STOCKBURGER JOEL 35 OLA AVE EAST HAMPTON, CT 06424

03A-44-C-106 TOZZI PAOLO 21 LAKEWOOD RD EAST HAMPTON, CT 06424

Attach plans showing all alternatives considered.
7. Attach a site plan showing the proposed activity and existing and proposed conditions in relation to wetlands and watercourses and identifying any further activities associated with, or reasonably related to, the proposed regulated activity which are made inevitable by the proposed regulated activity and which may have an impact on wetlands or watercourses. Include a colored grading plan showing areas to be filled (green) and areas to be excavated (brown) that clearly shows existing and proposed contours and proposed limits of disturbance.
8. Attach the names and mailing addresses of adjacent landowners. Attach additional sheets if necessary. NameAddress NameAddressAddress
9. Attach a completed DEEP reporting form. The Agency shall revise or correct the information provided by the applicant and submit the form to the Commissioner of Environmental Protection in accordance with section 22a-39-14 of the Regulations of Connecticut State Agencies.
10. Attach the appropriate filing fee based on the fee schedule in Section 19 of the regulations. Fee: _ (Make check payable to "The Town of East Hampton")
11. Name of Erosion Control Agent (Person Responsible for Compliance):
12. Are you aware of any wetland violations (past or present) on this property? YES NO If yes, explain
13. Are you aware of any vernal pools located on or adjacent (within 500')to the property? YES
14. For projects that do not fall under the ACOE Category 1 general permit – Have you contacted the Army Corps of Engineers? YES NO
15 Is this project within a public water supply aquifer protection area or a public water supply watershed area? YES NO If so, have you notified the Commissioner of the Connecticut Department of Public Health and the East Hampton WPCA? YES NO (Proof of notification must be submitted with your application.)
16. PUBLIC HEARINGS ONLY. The applicant must provide proof of mailing notices to the abutters prior to the hearing date.
17. As the applicant I am familiar with all the information provided in the application and I am aware of

Printed name: Will Me Med , Signature: ______, Date: ______, Date: ______, Date: _______, Date: ________, Please Note: You or a representative must attend the Inland Wetlands meeting to present you

information.

application.

SECTION 19 **APPLICATION FEES**

19.5 Fee Schedule. Application fees will be based on the following schedule:

	DEEP fee required by C.G.S. 22a-27j will be added to the base fee	\$60.00
19.5.1	Application Fee plus fee from Schedule A 19.5.1.1 Residential Uses. *Each additional lot with regulated activities. 19.5.1.2 Commercial/Industrial/Other Uses.	\$75.00 Plus *Plus \$50.00/lot \$400.00
19.5.2	Approval by Authorized Agent 19.5.2.1 Residential 19.5.2.2 Commercial	\$60.00 \$75.00
19.5.3	Public Hearing Fee 19.5.3.1 Single Residential 19.5.3.2 Subdivision 19.5.3.2 Commercial, Industrial, Other	\$100.00 \$400.00 \$400.00
19.5.4	Complex Application Fee The Inland Wetland Agency may charge an additional fee sufficient to cover to on complex applications. Such fee may include, but not be limited to, the cost review, and report on issues requiring such experts. The Agency shall estimate, which shall be paid pursuant to section 19 of these regulations within 10 d applicant's receipt or notice of such estimate. Any portion of the complex appl actual cost shall be refunded to the applicant no later than 30 days after public decision.	of retaining experts, to advise, ate the complex application ays of the ication fee in excess of the
19.5.5	Permitted and Nonregulated Uses: 19.5.5.1 Permitted Uses as of Right 19.5.5.2 Nonregulated	\$25.00 \$0.00
19.5.6	Regulation Amendment Petitions (Does not include Notices or Regulation Advisories from DEEP.) 19.5.6.1 Map Amendment Petitions Plus fee from Schedule B	\$150.00 \$50.00
19.5.7	Modification of Previous Approval 19.5.7.1 Residential 19.5.7.2 Subdivision 19.5.7.3 Commercial/Industrial/Other	\$ 25.00 \$ 50.00 \$ 75.00
19.5.8	Renewal of Previous Approval	\$50.00
19.5.9 area of	SCHEDULE A. For the purposes of calculating the permit application fee, the wetlands and watercourses and upland review area upon which a regulated a SQUARE FEET OF AREA	area in schedule A is the total ctivity is proposed.

SQUARE FEET OF AREA

19.5.9.1 Less than 1,000	\$0.00	
19.5.9.2 1,000 to 5,000	\$200.00)
19.5.9.3 More than 5,000	\$400.00)

19.5.10 SCHEDULE B. For the purposed of calculating the map amendment petition fee, the linear feet in schedule B is the total length of wetlands and watercourses boundary subject to the proposed boundary change.

1 11	VF.	ΔR	FF	EET	
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\$0.00
\$100.00
\$200.00

Town of East Hampton INLAND WETLANDS WATERCOURSE AGENCY 2024 Meeting Dates 1 Community Drive **Town Hall Council Chambers** 6:30 p.m.

Deadline:

2024

January 31, 2024	January 17, 2024
February 28, 2024	February 14, 2024
March 27, 2024	March 13, 2024

Meeting Date:

April 24, 2024 April 10, 2024

May 29, 2024 May 15, 2024

June 26, 2024 June 12, 2024

July 31, 2024 July 17, 2024

August 28, 2024 August 14, 2024

September 25, 2024 September 11, 2024

October 30, 2024 October 16, 2024

November 20, 2024 November 6, 2024

December 18, 2024 December 4, 2024

January 29, 2025 January 15, 2025

CHECKLIST FOR A COMPLETE APPLICATION	
☐ A narrative of the purpose and description and methodology of all preposed at the purpose and description and methodology of all preposed at the purpose and description and methodology of all preposed at the purpose and description and methodology of all preposed at the purpose and description and methodology of all preposed at the purpose and description and methodology of all preposed at the purpose and description and methodology of all preposed at the purpose and description and methodology of all preposed at the purpose and description and methodology of all preposed at the purpose and description and methodology of all preposed at the purpose and description and methodology of all preposed at the purpose and description and the purpose at the purpose a	
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wetlands. Such alternatives to be diagrammed on a site plan or drawing and submitted to the commission as part of the application:	stee
)f
□ Names and mailing addresses of abutting property owners;	
Three copies of approximately I"=40' scale plans	
Locations of existing and proposed land uses	
Locations of existing and proposed buildings	
Locations of existing and proposed subsurface sewage disposal systems, and test hole descriptions Existing and proposed topographical and man mode features in the state of t	
Existing and proposed topographical and man-made features including roads and driveways, on and adjacent to the site. Include a colored grading plan showing group to be filled to	
the site. Include a colored grading plan showing areas to be filled (green) and areas to be excavated (brown) that clearly shows existing and proposed contents.	
(brown) that clearly shows existing and proposed contours and proposed limits of disturbance.	
Location and diagrams of proposed erosion control structures	
Pictures of existing conditions clearly showing all arges to be distinct and the state of the st	
Pictures of existing conditions clearly showing all areas to be disturbed, and/or cleared of vegetation. Assessor map, block and lot number	
☐ Key or inset map	
□ North arrow	
□ Flood zone classification and delineation	
Use of wetland and watercourse markers where appropriate.	
Soil types classification and boundary delineation (flagged and more)	
□ Soil types classification and boundary delineation (flagged and numbered boundary), Soil Scientist's original signature and certification on plans	
Grand Continuation on Digita	
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and the location and now direction. Where appropriate	
and deploted on plans	
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200 curry, Sodding, or re-vegetation plans for all unprotected or un-vegetated areas	
== state and adolgh of structural sequinent control measures	
and a planting codificit collicit ties stills	
Use of wetland and watercourse markers Proper certification on the application documents and plans	
a report and another the application the tip and blane	
In the case of filling in wetlands, watercourses, or regulated upland areas, the following items are necessary:	
- " out to bo filled	
Trialitie of requested fill	
and the stape of third dicas	
□ Containment and stabilization measures □ Proposed finished contours	
Evaluation of the effect of filling the wotlands with recreat the translation of the effect of filling the wotlands with recreat the translation.	
need for flood control downstream	
Other required items:	
and a significant to the deficiency of the significant to the signific	
A written narrative detailing how the effects of the applicant's proposed activities upon wetlands and	
and all did all future plans will() may be linked to the activities proposed in the august	
and an an angle in total cation, with Lower Eligible Fillion	
☐ Mailing requirements for abutters (public hearing only)	

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Attach this card to the back of the mailpiece, or on the front if space permits.	B. Received by (Printed Name)	C: Date of Delivery
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Town of East Hampton Land Use Department

1 Community Dr., East Hampton, CT. 06424 (860) 267-7450

1	D	ECEIVE	Statement of the last of the l
1	7	MAY C occu	Ī

Inquiry/Complaint Form

Date Received:	Complaint #:					
Violator Information						
Location: Map#:_	Block#:Lot#:					
Property Owner:						
Mailing Address:						
Telephone (H):(W):(C):						
Complainant Information	May 6, 2024					
Name: Tamela Catalina Telephone (H):	(c): 860-463-0517					
Address: 37 LARE WOOD Rd Email: psalm 23 po	am@msv.com					
Signature: Hamila Catalena						
Complaint/Inquiry: For the First time in 28 years I now have a Stream of						
water Flowing From the Adjacent property west between my property						
and #39 Next door. I Flows strongly between	en our propertys					
across my drive way. I now have efflores	cence on my concrete					
Location on Property Where Violation is Occurring: Behind and						
house#37 and #39, Across myd						
my driveway						
Photograph Attached						
and the second residual and the second secon						
This section is to be completed by Planning and Zoning Staff.						
Refer to: Zoning Wetlands Building Blight	Health					
Pending / Recent Applications:						
Special Permits/ Exceptions:						
Inspector:Inspection Date:	J-					
Inspection Notes:						

From garage and my Furnace room For the First time ever. I am very concerned a boot my Foundation and water damage From this redirected Flow of water.

Jam a resident of East Hompton since 1996. I purchased my home in 1999. 28 years I have never seen this issue before.

of redireted water flow can be addressed.

Not sore if the tree removal behind my house is a direct result of new water. But seems to be related and I think can be remedied.

I know the town is addressing this concern Re: the development behind my house and I greatly appreciate your attention to this Desblem. It is also a concern that the excess Not to mention the problems caused by the tree removal that is also effecting our precious lake health, be concidered.

Thank you For your attention to this issue.

Sincerly,

Panela Catalina 37 LAKEWOOD Rd. East Hampton, CT 06424

Continued

may 6,200

one last concern I have is the milling that was done on Lakewood Rd. on the west side. I realize we had an ice issue down the entire road and the milling seemed to make that better however I would like to Know if there is anything else that can be done to reduce the runoff.

My driveway is on an extream angle and enterno my driveway now my care bottoms out. Very difficult.

Thouk you for your attention to this issue.

Sam Calaliner 37 LAKE Wood R. E. Hampton, CT 06434



PROJECT DESIGN REPORT

"Lake Overlook Estates"

Lakewood Road – Rear M 03A/B 44/L 23 East Hampton, Connecticut

Applicant/ Owner:

Lakewood Estates, LLC 244 Middletown Avenue East Hampton, CT 06424

Prepared By:

Robert V. Baltramaitis, P.E. 27 Tammy Hill Road Wallingford, CT 06492 (203) 915-8301 baltro@aol.com

April 17, 2024



PROJECT SUMMARY

The Applicant, Lakewood Estates, LLC, proposes to develop their property at Lakewood Road - Rear in East Hampton, Connecticut. The property is located along Lakewood Road with a 100-foot wide access way located approximately 1,200 feet from the intersection with Lake Drive. The parcel is 38.0 acres in size and slopes easterly and southeasterly towards Lakewood Road.

The applicant proposes to develop the property as an "Open Space Residential Subdivision" with eleven (11) building lots. The project will provide 7.7 acres of open space with this initial phase. A 13.87 acre parcel at the end of the proposed cul-de-sac roadway will remain for potential future development. The concept of the "Open Space Residential Subdivision" was preferred over a conventional subdivision by the East Hampton Planning and Zoning Commission at their meeting on February 7, 2024 during a pre-application discussion.

The proposed development will be served by a 700-foot long, 24-foot wide paved roadway with an adjacent 4-foot wide pedestrian sidewalk. These improvements will be constructed per Town of East Hampton standards and turned over to the Town as public improvements. Utilities to the site will come from Lakewood Road. Power and communications will come from the existing overhead lines along the west ide of Lakewood Road although they will be installed underground in the project area. Water will be provided from Aquarion Water Company of Connecticut and sanitary sewer will connect to the public system maintained by the East Hampton Water Pollution Control Authority.

A detailed Sediment & Erosion Control Plan has been developed to mitigate the short-term impacts of the development during construction. Overall, the S&E plan and the permanent storm water management system provide excellent protection and enhancement of storm water quality during and after construction.

A review of FEMA Flood Insurance Rate Map No. 09007C0135G, August 8, 2008, suggests that the site and immediate vicinity does NOT contain any flood hazard areas. Based on field reconnaissance and soil survey by James Sipperly, Certified Soil Scientist, there are no regulated wetlands or watercourses on the subject property.

DRAINAGE ANALYSIS

Hydraflow Hydrographs® (HH) software was used to generate computer models of the pre- and post-development scenarios. HH utilizes the methodologies set forth in the Technical Release No. 55 (TR-55) and Technical Release No. 20 (TR-20) computer model, originally developed by the Soil Conservation Service (SCS) now called the Natural Resources Conservation Service (NRCS). The HH software predicts runoff rates based upon several factors including land use, hydrologic soil type, vegetative cover, watershed area, time of concentration rainfall data and the attenuation effects due to ponds and structures. The SCS data is attached.

The site is presently undeveloped and entirely wooded. Presently, under *pre-development conditions*, the entire site drains overland from just below Highland Terrace easterly and southeasterly towards Lakewood Road. For analysis purposes, the pre-development drainage area is 25.4 acres and is broken down into three separate areas for analysis purposes. The area labeled 'ex-da-1' is 13.6 acres and drains to the southeast portion of the site. The area labeled as 'ex-da-2' is 10.1 acres drains to the middle part of the eastern boundary in the vicinity of the existing pump station. A third area labeled 'ex-da-3' is 1.7 acres and drains to the northeasterly portion of the site. While the site is slightly larger than the drainage study area, those areas are not being developed and do not drain towards Lakewood Road.

Under *post-development conditions*, the existing drainage areas will be altered. Proposed drainage area 1 (PR-DA-1) draining to the southeasterly portion of the site will be reduced in area from 13.6 acres to 7.1 acres. This area will see some site coverage alterations with development of portions of Lot #8 and Lot #9. Proposed drainage area 3 (PR-DA-3) will remain at 1.7 acres and see some alterations with the development of Lot #1.

The remaining area, drainage area 2 (PR-DA-2) will increase in size from 10.1 to 16.6 acres and see the bulk of the alterations. This drainage area is further broken down into PR-DA-2A consisting of 15.9 acres which drains to the proposed storm water management basin. A series of swales and storm pipe will collect runoff from the back sides of Lot #10, Lot #11 and Lot #12. This has been done intentionally so that the vast majority of storm water runoff is directed to and treated by the proposed storm water basin serving the site. This will help control peak runoff rates and enhance storm water quality. The remaining 0.7 acres is labeled as PR-DA-2B and is beyond the catchment area serving the detention basin and drains to the existing storm sewer system in Lakewood Road.

Based on existing land coverage in the sub watersheds and the underlying soils, as identified in the Soil Survey of New London County, Connecticut, runoff curve numbers (CN) were developed for each drainage area for existing and post-development conditions. The following runoff coefficients were used based on the hydrologic classification of area soils which are of hydrologic class 'B':

Land Cover	CN				
Roofs	98				
Pavements	98				
Grass (Fair)	69				
Grass (Good)	61				
Woods (Fair)	60				

Drainage areas for pre- and post-development scenarios were developed and are shown on the attached maps WS-1 and WS-2.

The times-of-concentration (Tc) were determined for both the pre- and post-development conditions using the SCS Lag Method given parameters of the watershed affecting overland flow, gutter flow, channel flow and pipe flow, where applicable. Given the impervious nature of

post development area 2B, a minimum time-of-concentration (Tc) of 5 minutes was utilized. Hydrographs were developed using SCS TR-55 methodology to ascertain flow rates and volumes, utilizing NOAA 14 published rainfall values. The associated 24-hour rainfall totals utilized are 3.38", 5.18", 6.30", 7.13" and 8.04" for the 2-, 10-, 25-, 50- and 100-year storms, respectively.

Not surprisingly, the developed site is anticipated to increase the peak runoff flow rates from the site. To mitigate this impact, a surface detention basin is proposed at the bottom reach of drainage area PR-DA-2A. This basin will have a sediment forebay and is designed as a dry basin; with the lowest outlet orifice invert matching that of the basin floor. Based on the proposed contours and configuration, the following table summarizes the stage/ storage relationship:

Stage	Elevation	Contour Area	Total Storage
(ft)	(ft)	(sq. ft.)	(cu. ft.)
0	532	8,500	0
1	533	10,110	9,292
2	534	11,830	20,250
3	535	13,610	32,958
4	536	15,650	47,531
5	537	17,475	64,038
6	538	19,900	82,710
7	539	24,000	104,626

The native soil classification is Charlton-Chatfield Complex which are well drained fine sandy loams and gravelly fine sandy loams with a Hydrologic classification of 'B'. Based on the physical properties of the sub-surface soils indicated by the Middlesex County Soil Survey, a conservative infiltration rate of 3 inches per hour was utilized. While this system will lose some nominal storm water volume to infiltration, the discharges are primarily controlled by the proposed outlet control structure. The system will ultimately discharge into the existing storm drainage system within Lakewood Road.

The following table summarizes the overall site runoff for pre-, post-developed (without detention), and post-developed (with detention) conditions for the design storms:

Storm	Pre-Developed	Post-Developed (no mitigation)	Post-Developed (w/ infiltration)		
2-year	6.0 cfs	9.3 cfs	4.6 cfs		
10-year	22.9 cfs	31.3 cfs	14.9 cfs		
25-year	36.3 cfs	47.6 cfs	22,5 cfs		
50-year	47.1 cfs	60.6 cfs	29.1 cfs		
100-year	59.5 cfs	75.5 cfs	35.7 cfs		

As summarized above and in the calculations contained in the Appendix, with provision of the proposed storm water management basin, the post-development peak runoff flows will be effectively reduced to BELOW pre-development levels. Drainage calculations are attached.

STORM WATER QUALITY

Several storm water quality measures or Best Management Practices (BMP's) have been incorporated into the storm water management system to maintain storm water quality both during construction and after permanent vegetation has been established. Storm water quality measures have been implemented in accordance with the CT DEEP Stormwater Quality Manual (2004).

The first line defense along the water treatment train is the catch basins themselves. Catch basins will be constructed with 2-foot deep sumps to trap coarse sediments. Before final discharge, storm water collected from the proposed paved areas will pass through structural water quality enhancement units (oil/ grit separator). Storm water from the roof areas will be collected and discharged separately into U/G infiltration systems to increase the effectiveness of the structural units. Clean rooftop water will recharge the ground water table. After treatment by the units, storm water will enter a sediment forebay of the detention basin to provide extended contact time with vegetation, an important measure to remove heavy metals and other pollutants.

SEDIMENT & EROSION CONTROL

A detailed Sediment & Erosion Control Plan has been developed to mitigate the short-term impacts of the development during construction. The S&E Control Plan includes descriptive specifications concerning land grading, topsoil application, temporary vegetative cover, permanent vegetative cover, vegetative cover selection, mulching and erosion checks. Details have been provided for all erosion controls with corresponding labels on the Sediment & Erosion Control Plan. All sediment and erosion controls are intended to be in full compliance with the Connecticut Guidelines for Soil Erosion and Sediment Control Manual, 2000, DEP Bulletin 34.

Geotextile sediment fence is proposed at the down-gradient limits of all construction activity. Along critical areas, the silt fence will be backed-up by staked haybales. A Temporary Sediment Basin is proposed to collect runoff from the site during construction and allow particles to settle out. Check dams will be provided, as necessary. An anti-tracking apron will be installed at the project entrances.

Overall, the S&E plan and the permanent storm water management system provide excellent protection and enhancement of storm water quality during and after construction.

APPENDIX

"Lake Overlook Estates"

Lakewood Road East Hampton, Connecticut

Web Soil Survey National Cooperative Soil Survey

Natural Resources Conservation Service

USDA

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres In AOI	Percent of AOI		
3	Ridgebury, Leicester, and Whitman soils, 0 to 8 percent slopes, extremely stony	6.1	4.2%		
50B	Sutton fine sandy loam, 3 to 8 percent slopes	1.6	1.1%		
58C	Gloucester gravelly sandy loam, 8 to 15 percent slopes, very stony	9.1	6.3%		
60C	Canton and Charlton fine sandy loams, 8 to 15 percent slopes	2.6	1.8%		
62D	Canton and Charlton fine sandy loams, 15 to 35 percent slopes, extremely stony		0,0%		
73C	Chariton-Chatfield complex, 0 to 15 percent slopes, very rocky	37.0	25.5%		
73E	Charlton-Chalfield complex, 15 to 45 percent slopes, very rocky	66.3	45.8%		
84C	Paxton and Montauk fine sandy loams, 8 to 15 percent slopes	0.4	0.3%		
85B	Paxton and Montauk fine sandy loams, 3 to 8 percent slopes, very stony	2,2	1.5%		
86D	Paxton and Montauk fine sandy loams, 15 to 35 percent slopes, extremely stony	8.1	5.6%		
229B	Agawam-Urban land complex, 0 to 8 percent slopes	9.2	6,3%		
273E	Urban land-Charlton-Chatfield complex, rocky, 15 to 45 percent slopes	0.2	0.1%		
305	Udorthents-Pits complex, gravelly	1.2	0.8%		
W	Water	1.0	0.7%		
Totals for Area of Interest	,	144.8	100.0%		



NOAA Atlas 14, Volume 10, Version 3 Location name: Town of East Hampton, Connecticut, USA*

Latitude: 41.5811°, Longitude: -72.4811° Elevation: 490 ft** source: ESRI Maps



POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sandra Pavlovic, Michael St. Laurent, Carl Trypaluk, Dale Unruh, Orlan Wilhite

NOAA, National Weather Service, Silver Spring, Maryland

PF tabular | PF graphical | Maps & aerials

PF tabular

PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches) ¹												
Duration				Average	recurrence	interval (ye	ears)					
	1	2	5	10	25	50	100	200	500	1000		
5-min	0.334 (0.259-0.421)	0.405 (0.314-0.511)	0.521 (0.402-0.659)	0.617 (0.474-0.784)	0.750 (0.558-0.994)	0.850 (0.619-1.15)	0.954 (0.677-1.34)	1.07 (0.720-1.53)	1.24 (0.803-1.83)	1.38 (0.872-2.07)		
10-min	0.474	0.574	0.738 (0.569-0.933)	0.874	1.06	1.20	1.35	1.52	1.76 (1.14-2.59)	1.95 (1.24-2.94)		
15-min	0.557 (0.432-0.702)	0.676 (0.523-0.851)	0.870 (0.670-1.10)	1.03 (0.791-1.31)	1.25 (0.930-1.66)	1.42 (1.03-1.92)	1,59 (1.13-2.23)	1.79 (1.20-2.55)	2.07 (1.34-3.05)	2.30 (1.45-3.46)		
30-min	0.761 (0.590-0.958)	0.923 (0.714-1.16)	1.19 (0.916-1.50)	1.40 (1.08-1.78)	1.71 (1.27-2.26)	1.93 (1.41-2.61)	2.17 (1.54-3.04)	2,44 (1.64-3.48)	2.82 (1.83-4.16)	3.13 (1.98-4.71)		
60-min	0.965 (0.748-1.22)	1.17 (0.905-1.47)	1.50 (1.16-1.90)	1.78 (1.37-2.26)	2.16 (1.61-2.86)	2.45 (1.78-3.31)	2.75 (1.95-3.85)	3.09 (2.08-4.41)	3.57 (2.31-5,27)	3.97 (2.51-5.97)		
2-hr	1.27 (0.994-1.59)	1.53 (1.19-1.91)	1.95 (1.52-2.45)	2.30 (1.78-2.90)	2.78 (2.08-3.66)	3.14 (2.31-4.22)	3.52 (2.52-4.91)	3.96 (2.68-5.62)	4.62 (3.00-6.76)	5.17 (3.28-7.71)		
3-hr	1.48 (1.16-1.85)	1.78 (1.40-2.22)	2.26 (1.77-2.83)	2.66 (2.07-3.34)	3.21 (2.42-4.22)	3.62 (2.68-4.86)	4.06 (2.92-5.65)	4.58 (3.10-6.46)	5.36 (3.49-7.80)	6.01 (3.82-8.92)		
6-hr	1.90 (1.50-2,35)	2.27 (1.80-2,82)	2.89 (2.28-3.59)	3.40 (2.66-4.24)	4.10 (3.11-5.35)	4.62 (3.44-6.16)	5.18 (3,75-7,17)	5.85 (3.98-8.19)	6.86 (4.48-9.91)	7.72 (4.92-11.4)		
12-hr	2,36 (1.88-2.90)	2.84 (2.26-3.49)	3.63 (2.88-4.47)	4.28 (3.38-5.30)	5.17 (3.96-6.70)	5.84 (4.38-7.72)	6.56 (4.78-9.00)	7.41 (5.06-10.3)	8.70 (5.70-12.5)	9.79 (6.27-14.3)		
24-hr	2,78 (2,24-3,39)	3.38 (2.72-4.13)	4.36 (3.50-5.34)	5.18 (4.12-6.38)	6.30 (4.86-8.12)	7.13 (5,39-9.38)	8.04 (5.91-11.0)	9.13 (6.26-12.6)	10.8 (7.11-15,4)	12.2 (7.86-17.7)		
2-day	3.12 (2.53-3.78)	3.85 (3.12-4.66)	5.04 (4.07-6.13)	6.03 (4.84-7.36)	7.39 (5.74-9.47)	8.38 (6.39-11.0)	9.48 (7.05-13.0)	10.9 (7.48-14.9)	13.0 (8.60-18.4)	14.9 (9.62-21.4)		
3-day	3.38 (2.76-4.08)	4.18 (3.41-5.05)	5.49 (4.46-6.65)	6.57 (5.30-8.00)	8.06 (6.30-10.3)	9.15 (7.01-12.0)	10.4 (7.73-14.1)	11.9 (8.20-16.2)	14.3 (9.46-20.1)	16.4 (10.6-23.4)		
4-day	3.63 (2.97-4.37)	4.48 (3.66-5.39)	5.86 (4.78-7.08)	7.02 (5.68-8.51)	8.60 (6.74-10.9)	9.76 (7.50-12.7)	11.0 (8.26-15.0)	12.7 (8.75-17.2)	15.2 (10.1-21.3)	17.5 (11.3-24.8)		
7-day	4.31 (3.56-5.16)	5.26 (4.33-6.30)	6.81 (5.59-8.18)	8.10 (6.60-9.77)	9.87 (7.78-12.5)	11.2 (8.62-14.4)	12.6 (9.45-16.9)	14.4 (9.98-19.4)	17.2 (11.4-23.8)	19.6 (12.7-27.7)		
10-day	5.00 (4.14-5.96)	6.01 (4.97-7.16)	7.65 (6.30-9.15)	9.01 (7.37-10.8)	10.9 (8.60-13.7)	12,3 (9.48-15,7)	13.8 (10.3-18.3)	15.6 (10.9-20.9)	18.5 (12.3-25.5)	20.9 (13.6-29.3)		
20-day	7,17 (5.99-8.48)	8.25 (6.88-9.77)	10.0 (8.32-11.9)	11.5 (9.48-13.7)	13.5 (10.7-16.7)	15.0 (11.6-18.9)	16.6 (12.4-21.6)	18.4 (12.9-24.4)	21,0 (14.1-28.7)	23.1 (15.0-32.2)		
30-day	9.00 (7.56-10.6)	10.1 (8.49-11.9)	11.9 (9.98-14.1)	13.5 (11.2-16.0)	15.6 (12.4-19.1)	17.1 (13.3-21.4)	18.8 (14.0-24.1)	20.5 (14.4-27.0)	22.8 (15.3-31.0)	24.6 (16.1-34.1)		
45-day	11.3 (9.53-13.2)	12.5 (10.5-14.6)	14.3 (12.0-16.9)	15.9 (13.3-18.8)	18.1 (14.5-22.0)	19.8 (15.4-24.5)	21.4 (15.9-27.2)	23.0 (16.3-30.2)	25.1	26.6 (17.4-36.6)		
60-day	13.2 (11,2-15,4)	14.4 (12.2-16.9)	16.4 (13.8-19.2)	18.0 (15.1-21.2)	20.2 (16.2-24.5)	22.0 (17.1-27.0)	23.7 (17.6-29.8)	25.2 (17.9-32.9)	27.1	28.4 (18.5-38.9)		

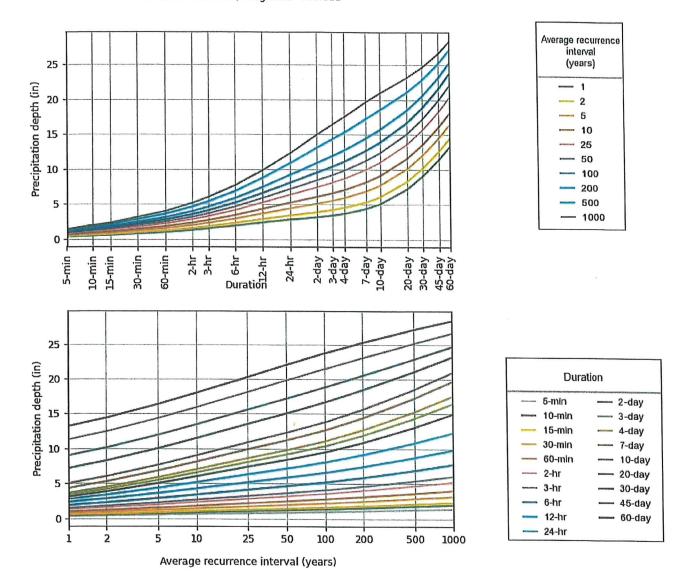
Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values. Please refer to NOAA Atlas 14 document for more information.

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PF graphical

PDS-based depth-duration-frequency (DDF) curves Latitude: 41.5811°, Longitude: -72.4811°



NOAA Atlas 14, Volume 10, Version 3

Created (GMT): Fri Aug 18 20:28:12 2023

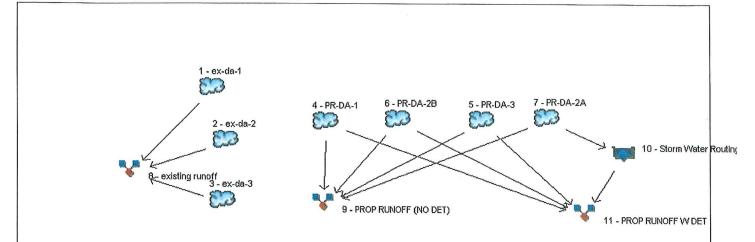
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Maps & aerials

Small scale terrain

Watershed Model Schematic

Hydraflow Hydrographs by Intelisolve v9.02



Legend

Hyd.	Origin	Description		
1	SCS Runoff	ex-da-1		
2	SCS Runoff	ex-da-2		
3	SCS Runoff	ex-da-3		
4	SCS Runoff	PR-DA-1		
5	SCS Runoff	PR-DA-3		
6	SCS Runoff	PR-DA-2B		
7	SCS Runoff	PR-DA-2A		
8	Combine	existing runoff		
9	Combine	PROP RUNOFF (NO DET)		
10	Reservoir	Storm Water Routing		
11	Combine	PROP RUNOFF W DET		
Proj	ject: storm r	nodel.gpw	Tuesday, Apr 16, 2024	

Hydrograph Return Period Recap

Hydraflow Hydrographs by Intelisolve v9.02

Hyd. Hydrograph Inflow Peak Outflow (cfs)								Hydraflow Hydrographs by Intelisolve v9.02			
Hyd. No.	Hydrograph type (origin)	Inflow Hyd(s)	1-Yr	2-Yr	3-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr	Hydrograph description
1	SCS Runoff			3.327		Nav	12.60	19.91	25,76	32,60	ex-da-1
2	SCS Runoff	202000		2.365			8.968	14.15	18.34	23.15	ex-da-2
3	SCS Runoff			0.520			2.097	3.326	4.310	5.444	ex-da-3
4	SCS Runoff			1,957	******		7.002	10.90	14.03	17.65	PR-DA-1
5	SCS Runoff			0.582			1.881	2,854	3.635	4,527	PR-DA-3
6	SCS Runoff			0.490	L		1.426	2.098	2.623	3,217	PR-DA-2B
7	SCS Runoff			7.164		uu	23.03	34.73	43.95	54.46	PR-DA-2A
8	Combine	1, 2, 3,		6.001	-070555		22.95	36.35	47.08	59,48	existing runoff
9	Combine	4, 5, 6, 7,		9.333			31,29	47.61	60.60	75.52	PROP RUNOFF (NO DET)
10	Reservoir	7	V	2.822			9.165	12.86	15.79	18.27	Storm Water Routing
11	Combine	4, 5, 6, 10		4.624			14.87	22.53	29.15	35.71	PROP RUNOFF W DET
Proi	j. flle: storm n	nodel.apw	<i>,</i>						Tu	esdav. A	Apr 16, 2024

Hydrograph Summary Report

Hydraflow Hydrographs by Intelisolve v9.02

	-						Hydraflow Hydrographs by Intelisolve v9.02				
Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time Interval (min)	Time to peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph description		
1	SCS Runoff	32.60	2	738	163,448				ex-da-1		
2	SCS Runoff	23.15	2	742	123,311				ex-da-2		
3	SCS Runoff	5,444	2	730	20,431			u-u	ex-da-3		
4	SCS Runoff	17.65	2	738	88,167			y	PR-DA-1		
5	SCS Runoff	4.527	2	738	22,477	2525			PR-DA-3		
6	SCS Runoff	3.217	2	724	9,625			concen	PR-DA-2B		
7	SCS Runoff	54.46	2	732	220,085				PR-DA-2A		
8	Combine	59.48	2	738	307,190	1, 2, 3,			existing runoff		
9	Combine	75.52	2	732	340,353	4, 5, 6, 7,			PROP RUNOFF (NO DET)		
10	Reservoir	18.27	2	754	190,503	7	537.53	73,792	Storm Water Routing		
11	Combine	35.71	2	740	304,003	4, 5, 6, 10		u	PROP RUNOFF W DET		
stor	m model.gpw			****	Return P	eriod: 100	Year	Tuesday, A	hpr 16, 2024		

Hydraflow Hydrographs by Intelisolve v9.02

Tuesday, Apr 16, 2024

Hyd. No. 1

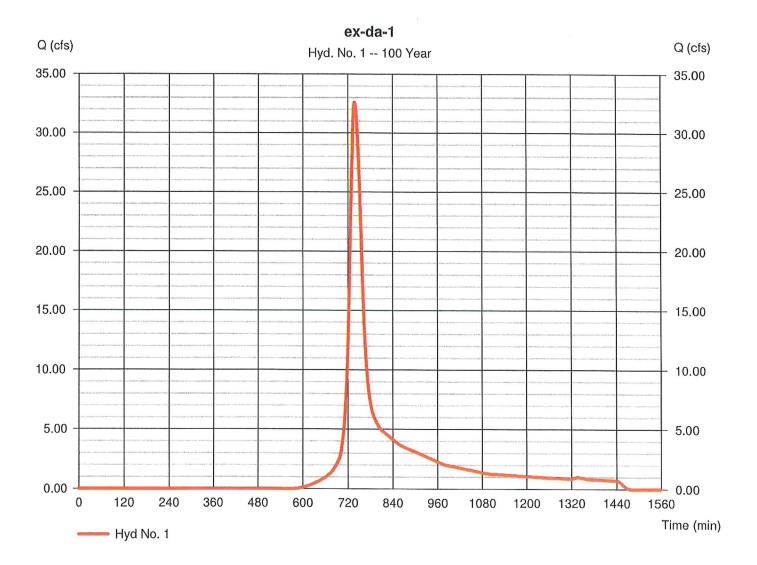
ex-da-1

Hydrograph type = SCS Runoff Storm frequency = 100 yrsTime interval $= 2 \min$ Drainage area = 13.600 acBasin Slope = 0.0 %Tc method = TR55 Total precip. = 8.04 inStorm duration = 24 hrs

Peak discharge = 32.60 cfs Time to peak = 738 min Hyd. volume = 163,448 cuft Curve number = 60

Hydraulic length = 0 ft
Time of conc. (Tc) = 25.50 min
Distribution = Type III

Shape factor = 484



Hydraflow Hydrographs by Intelisolve v9.02

Hyd. No. 1

ex-da-1

Description	<u>A</u>		B		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 300.0 = 3.38 = 15.00		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 22.47	+	0.00	+	0.00	=	22.47
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 1125.00 = 15.00 = Unpaved = 6.25		0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 3.00	+	0.00	+	0.00	=	3.00
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s) Flow length (ft)	= 0.00 = 0.00 = 0.00 = 0.015 = 0.00 = 0.0		0.00 0.00 0.00 0.015 0.00 0.0		0.00 0.00 0.00 0.015 0.00		
Travel Time (min)	= 0.00	+	0.00	+	0.00		0.00
Total Travel Time, Tc	**************	*******					25.50 min

Hydraflow Hydrographs by Intelisolve v9.02

Tuesday, Apr 16, 2024

Hyd. No. 2

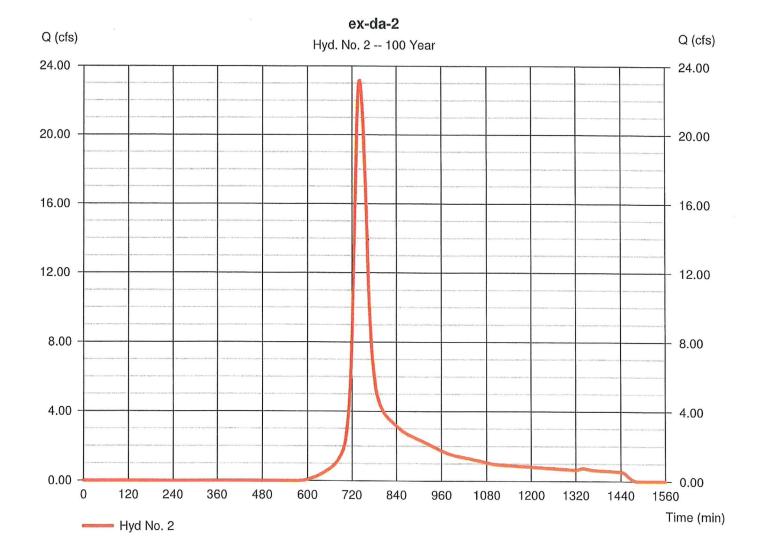
ex-da-2

Hydrograph type = SCS Runoff Storm frequency = 100 yrsTime interval = 2 min Drainage area = 10.100 acBasin Slope = 0.0 %Tc method = TR55 Total precip. = 8.04 inStorm duration = 24 hrs

Peak discharge = 23.15 cfs
Time to peak = 742 min
Hyd. volume = 123,311 cuft

Curve number = 60 Hydraulic length = 0 ft Time of conc. (Tc) = 27.10 min

Distribution = 7.10 min Shape factor = 484



Hydraflow Hydrographs by Intelisolve v9.02

Hyd. No. 2

ex-da-2

<u>Description</u>	<u>A</u>		B		<u>C</u>		<u>Totals</u>			
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 300.0 = 3.38 = 12.70		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00					
Travel Time (min)	= 24.02	+	0.00	+	0.00	=	24.02			
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 1070.00 = 12.70 = Unpaved = 5.75		0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00					
Travel Time (min)	= 3.10	+	0.00	+	0.00	=	3.10			
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s) Flow length (ft)	= 0.00 = 0.00 = 0.00 = 0.015 = 0.00 = 0.0		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015 0.00					
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00			
Total Travel Time, Tc										

Hydraflow Hydrographs by Intelisolve v9.02

Tuesday, Apr 16, 2024

Hyd. No. 3

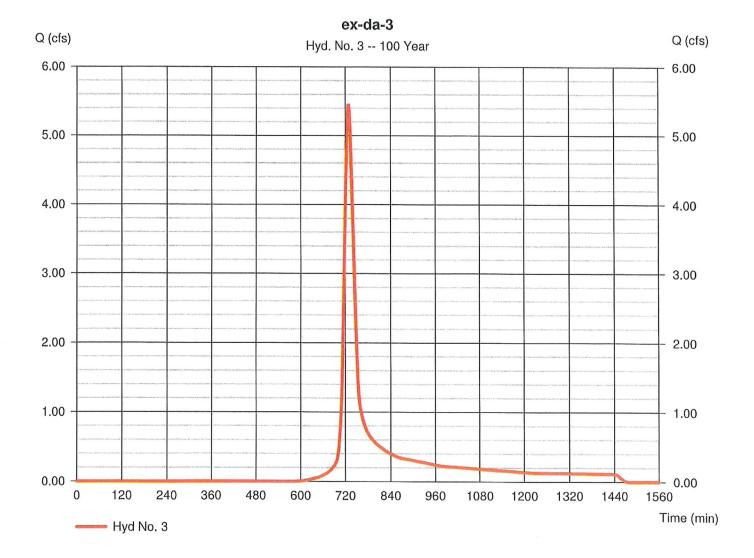
ex-da-3

Hydrograph type = SCS Runoff Storm frequency = 100 yrsTime interval = 2 min Drainage area = 1.700 acBasin Slope = 0.0 %Tc method = TR55 Total precip. = 8.04 inStorm duration = 24 hrs

Peak discharge = 5.444 cfs Time to peak = 730 min Hyd. volume = 20,431 cuft

Curve number = 60Hydraulic length = 0 ft

Time of conc. (Tc) = 23.70 min
Distribution = Type II
Shape factor = 484



Hydraflow Hydrographs by Intelisoive v9.02

Hyd. No. 3

ex-da-3

Description		<u>A</u>		В		<u>c</u>		<u>Totals</u>		
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	=	0.400 300.0 3.38 14.50		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00				
Travel Time (min)	=	22.78	+	0.00	+	0.00	=	22.78		
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	=	350.00 14.50 Unpaved 6.14		0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00				
Travel Time (min)	=	0.95	+	0.00	+	0.00	-	0.95		
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s) Flow length (ft)	11 11 11	0.00 0.00 0.00 0.015 0.00 0.0		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015 0.00 0.0				
Travel Time (min)	=	0.00	+	0.00	+	0.00	=	0.00		
Total Travel Time, Tc	Total Travel Time, Tc									

Hydraflow Hydrographs by Intelisolve v9.02

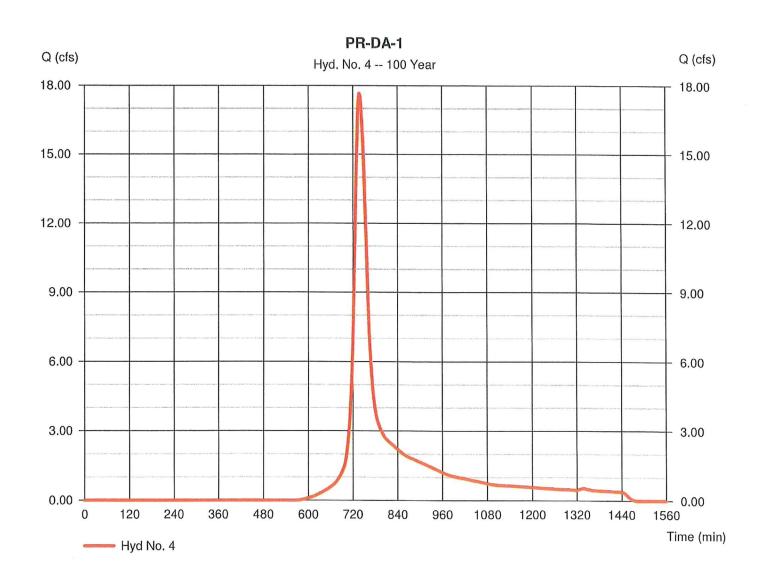
Tuesday, Apr 16, 2024

Hyd. No. 4

PR-DA-1

Hydrograph type = SCS Runoff Peak discharge = 17.65 cfsStorm frequency = 100 yrsTime to peak = 738 min Time interval = 2 min Hyd. volume = 88,167 cuft = 7.100 acDrainage area Curve number = 61* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 25.50 minTotal precip. Distribution = 8.04 in= Type III Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(5.900 x 60) + (0.200 x 98) + (1.000 x 61)] / 7.100



Hydraflow Hydrographs by Intelisolve v9.02

Hyd. No. 4

PR-DA-1

<u>Description</u>	<u>A</u>		<u>B</u>		<u>c</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 300.0 = 3.38 = 15.00		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 22.47	+	0.00	+	0.00	=	22.47
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 1125.00 = 15.00 = Unpaved = 6.25	i	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 3.00	+	0.00	+	0.00	Ħ	3.00
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s) Flow length (ft)	= 0.00 = 0.00 = 0.00 = 0.015 = 0.00 = 0.0		0.00 0.00 0.00 0.015 0.00 0.0		0.00 0.00 0.00 0.015 0.00 0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00		0.00
Total Travel Time, Tc	9 P 9 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8						25.50 min

Hydraflow Hydrographs by Intelisolve v9.02

Tuesday, Apr 16, 2024

Hyd. No. 5

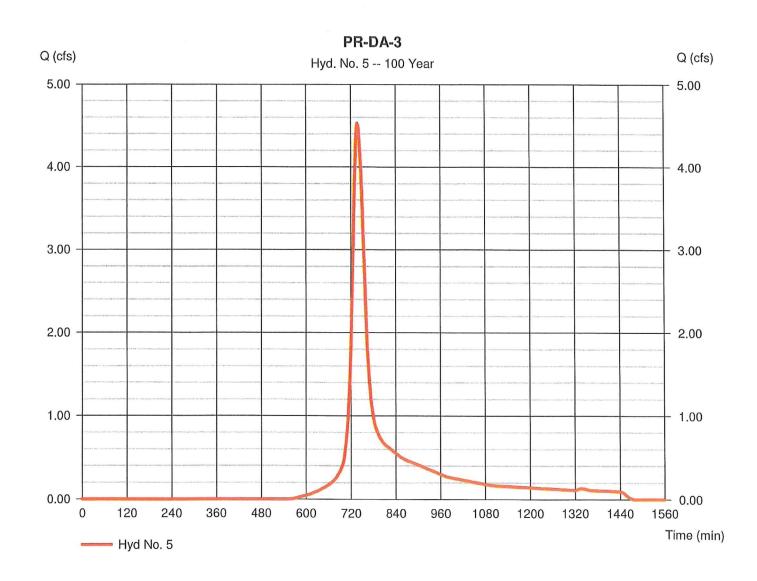
PR-DA-3

Hydrograph type = SCS Runoff Storm frequency = 100 yrsTime interval $= 2 \min$ Drainage area = 1.700 acBasin Slope = 0.0 %Tc method = TR55 Total precip. = 8.04 inStorm duration = 24 hrs

Peak discharge = 4.527 cfs Time to peak = 738 min Hyd. volume = 22,477 cuft Curve number = 63*

Hydraulic length = 0 ft
Time of conc. (Tc) = 23.70 min
Distribution = Type III
Shape factor = 484

^{*} Composite (Area/CN) = [(1.100 x 60) + (0.100 x 98) + (0.500 x 61)] / 1.700



Hydraflow Hydrographs by Intelisolve v9.02

Hyd. No. 5

PR-DA-3

<u>Description</u>	<u> </u>	<u>A</u>		В		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 3 = 3	0.400 300.0 3.38 14.50		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 2	22.78	+	0.00	+	0.00	=	22.78
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 1 = (350.00 14.50 Jnpaved 3.14		0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= (0.95	+	0.00	+	0.00	=	0.95
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s) Flow length (ft)	= 0 = 0 = 0	0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015 0.00		
Travel Time (min)	= 0	0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc						23.70 min		

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= Type III

= 484

Hyd. No. 6

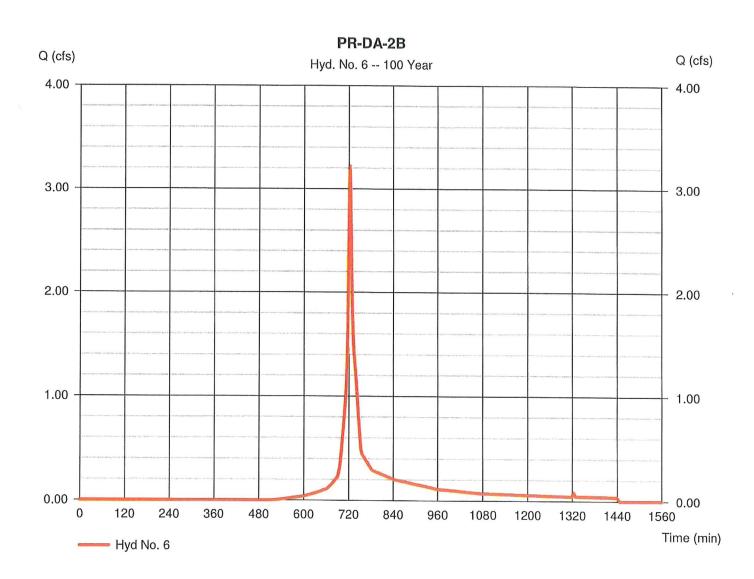
PR-DA-2B

Hydrograph type = SCS Runoff Storm frequency = 100 yrsTime interval = 2 minDrainage area = 0.700 acBasin Šlope = 0.0 %Tc method = USER Total precip. = 8.04 inStorm duration = 24 hrs

Peak discharge = 3.217 cfsTime to peak = 724 min Hyd. volume = 9,625 cuftCurve number = 66* Hydraulic length = 0 ftTime of conc. (Tc) = 5.00 minDistribution

Shape factor

* Composite (Area/CN) = $[(0.100 \times 98) + (0.300 \times 61) + (0.300 \times 60)] / 0.700$



Hydraflow Hydrographs by Intelisolve v9.02

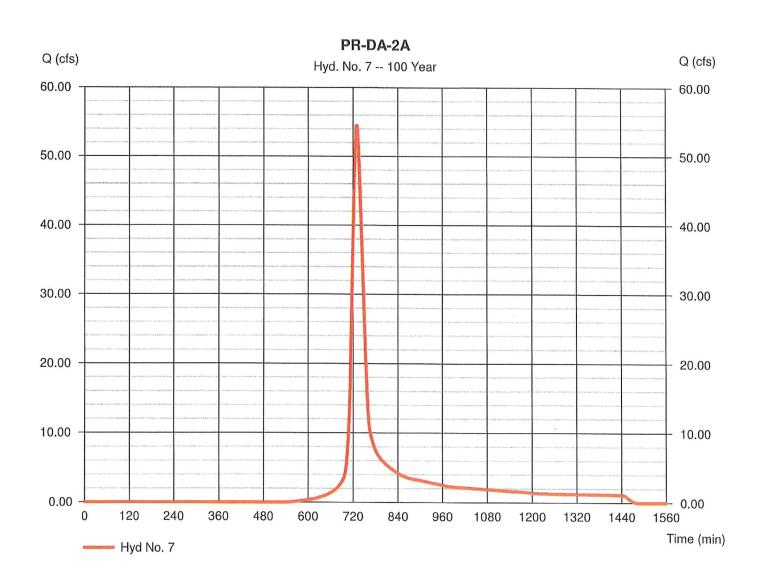
Tuesday, Apr 16, 2024

Hyd. No. 7

PR-DA-2A

= SCS Runoff Hydrograph type Peak discharge = 54.46 cfsStorm frequency = 100 yrsTime to peak = 732 min Time interval = 2 min Hyd. volume = 220,085 cuft Drainage area = 15.900 acCurve number = 64* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 27.20 minTotal precip. = 8.04 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(1.600 \times 98) + (4.500 \times 61) + (9.800 \times 60)] / 15.900$



Hydraflow Hydrographs by Intelisolve v9.02

Hyd. No. 7

PR-DA-2A

Description		<u>A</u>		<u>B</u>		<u>C</u>		Totals
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	11 11	0.400 300.0 3.38 12.70		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	=	24.02	+	0.00	+	0.00	=	24.02
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	11	540.00 12.70 Unpaved 5.75	[370.00 10.00 Paved 6.43		50.00 1.00 Unpave 1.61	ed	
Travel Time (min)	=	1.57	+	0.96	+	0.52	=	3.04
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s) Flow length (ft)		2.40 3.90 2.00 0.015 10.15 110.0		0.00 0.00 0.00 0.015 0.00 0.0		0.00 0.00 0.00 0.015 0.00		
Travel Time (min)	=	0.18	+	0.00	+	0.00	=	0.18
Total Travel Time, Tc						27.20 min		

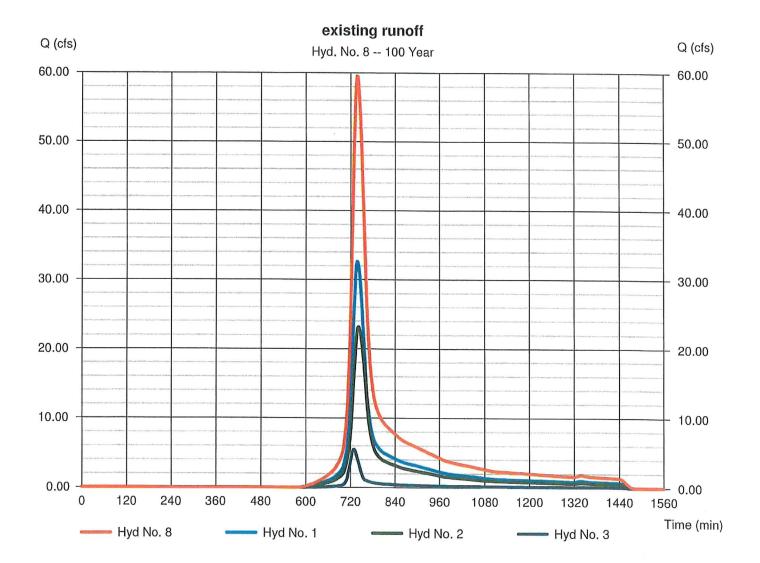
Hydraflow Hydrographs by Intelisolve v9.02

Tuesday, Apr 16, 2024

Hyd. No. 8

existing runoff

Hydrograph type = Combine Storm frequency = 100 yrs Time interval = 2 min Inflow hyds. = 1, 2, 3 Peak discharge = 59.48 cfs Time to peak = 738 min Hyd. volume = 307,190 cuft Contrib. drain. area = 25.400 ac



Hydraflow Hydrographs by Intelisolve v9.02

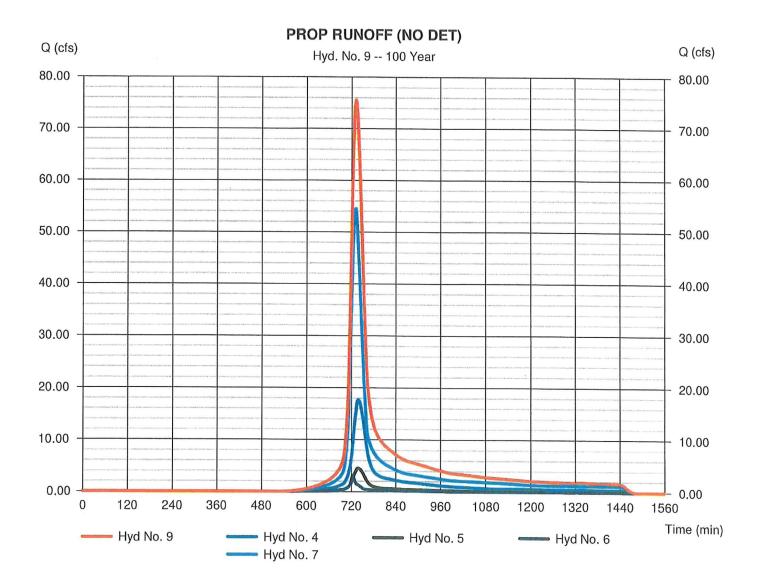
Tuesday, Apr 16, 2024

Hyd. No. 9

PROP RUNOFF (NO DET)

Hydrograph type = Combine Storm frequency = 100 yrs Time interval = 2 min Inflow hyds. = 4, 5, 6, 7

Peak discharge = 75.52 cfs Time to peak = 732 min Hyd. volume = 340,353 cuft Contrib. drain. area = 25.400 ac



Hydraflow Hydrographs by Intelisolve v9.02

Tuesday, Apr 16, 2024

Hyd. No. 10

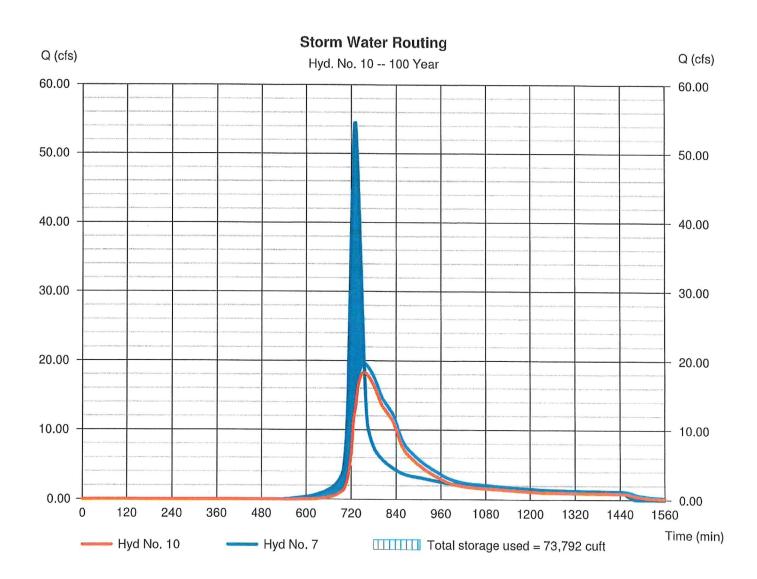
Storm Water Routing

Hydrograph type = Reservoir Storm frequency = 100 yrs Time interval = 2 min

Inflow hyd. No. = 7 - PR-DA-2A Reservoir name = DETENTION BASIN Peak discharge Time to peak = 18.27 cfs = 754 min

Hyd. volume = 190,503 cuft Max. Elevation = 537.53 ft Max. Storage = 73,792 cuft

Storage Indication method used. Exfiltration extracted from Outflow.



Pond Report

Hydraflow Hydrographs by Intelisolve v9.02

Tuesday, Apr 16, 2024

Pond No. 1 - DETENTION BASIN

Pond Data

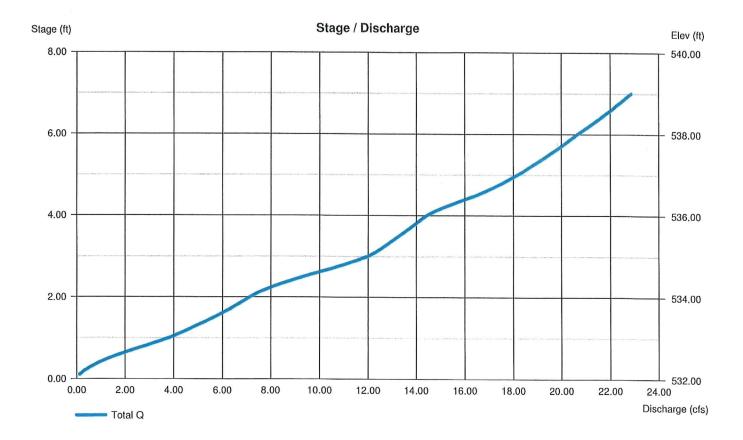
Contours - User-defined contour areas. Conic method used for volume calculation. Begining Elevation = 532.00 ft

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	532.00	8,500	0	0
1.00	533.00	10,110	9,292	9,292
2.00	534.00	11,830	10,958	20,250
3.00	535.00	13,610	12,708	32,958
4.00	536.00	15,560	14,573	47,531
5.00	537.00	17,475	16,507	64,038
6.00	538.00	19,900	18,673	82,710
7.00	539.00	24,000	21,916	104,626

Culvert / Orifice Structures Weir Structures [A] [B] [C] [PrfRsr] [A] [B] [C] [D] Rise (in) = 18.00 12.00 12.00 0.00 Crest Len (ft) = 4.00 0.00 10.00 0.00 Span (in) = 18.0012.00 12.00 0.00 Crest El. (ft) = 536.00 538.00 0.00 0.00 No. Barrels = 1 2 2 0 Weir Coeff. = 3.332.60 3.33 3.33 Invert El. (ft) = 532.00 532.00 534.00 0.00 Weir Type = Rect Broad Length (ft) = 50.000.00 0.00 0.00 Multi-Stage = Yes Yes No No Slope (%) = 2.000.00 0.00 n/a N-Value = .013.013 .013 n/a Orifice Coeff. = 0.600.60 0.60 0.60 Exfil.(in/hr) = 3.000 (by Contour) Multi-Stage Yes = n/aYes No TW Elev. (ft) = 0.00

Note: Culvert/Orifice outflows are analyzed under inlet and outlet control. Weir risers are checked for orifice conditions.



Hydraflow Hydrographs by Intelisolve v9.02

Tuesday, Apr 16, 2024

Hyd. No. 11

PROP RUNOFF W DET

Hydrograph type = Combine Storm frequency = 100 yrs Time interval = 2 min Inflow hyds. = 4, 5, 6, 10 Peak discharge = 35.71 cfs Time to peak = 740 min Hyd. volume = 304,003 cuft Contrib. drain. area = 9.500 ac

