RECEIVED 8.28.2023 East Hampton Land Use Office

## STORM WATER MANAGEMENT REPORT

## **Home Acres Estates**

**Proposed 15-Lot Residential Subdivision** 

Map 26/ Block 87/ Lot 6
Flanders Road
East Hampton, Connecticut

Applicant/ Owner: Flanders Road Estates, LLC 244 Middletown Avenue East Hampton, CT 06424

Prepared By:
Robert V. Baltramaitis, P.E.
27 Tammy Hill Road

Wallingford, CT 06492 (203) 915-8301

baltro@aol.com

August 2023



The Applicant, Home Acres Estates, LLC proposes to develop its property along Flanders Road in East Hampton. The parcel is known as M26B87L6 and is 52.9 acres in size. The parcel is located along the south side of Flanders Road east of Smith Street and just west of Bear Swamp Road (southern portion). The site location is depicted in attached Figure 1.

The parcel is currently zoned R-4 residential. The applicant proposes to subdivide the parcel into fifteen (15) conforming residential building lots. Three (3) of the lots will have access from the end of Pecausett Trail at the east side of the parcel. The remining twelve (12) lots will have access from a new 1,400-foot long paved roadway from Flanders Road; proposed to be privately owned. The private road will be 22-feet in width and be constructed with enough drainage amenities to collect and contain anticipated storm water runoff flows. Utilities serving the proposed lots will be installed below ground.

#### STORM WATER ANALYSIS

Hydraflow Hydrographs® (HH) software was used to generate computer models of the pre- and post-development scenarios. HH utilizes the methodologies set forth in the Technical Release No. 55 (TR-55) and Technical Release No. 20 (TR-20) computer model, originally developed by the Soil Conservation Service (SCS) now called the Natural Resources Conservation Service (NRCS). The HH software predicts runoff rates based upon several factors including land use, hydrologic soil type, vegetative cover, watershed area, time of concentration rainfall data and the attenuation effects due to ponds and structures.

While the parcel is 52.9 acres in size, it is part of a slightly larger watershed that extends easterly and includes a small portion of the Pecausett Drive subdivision. The watershed studied as part of this report, includes this area and totals 59.8 acres in size.

Presently, under *pre-development conditions*, almost the entire site is wooded with exception of approximately 2 acres that is cleared adjacent to the entrance at Flanders Road. The site presently drains primarily from east to west towards a large off-site wetland system adjacent to the linear trail. Based on existing topography, the site has two main drainage divides. Drainage Divide #1 consists of 25.9 acres and drains westerly and northwesterly through an on-site wetland system (wetland flags 14A through 49A) until it leaves the site.

The second Drainage Divide #2 consists of 33.9 acres and drains westerly. Based on the site topography, for analysis purposes, this drainage divide is further broken into smaller subwatersheds. Existing sub-watershed area 'ex-da-2a' drains westerly through a second on-site wetland system (wetland flags 1 through 32) until it leaves the site. Other drainage areas 'ex-da-2b' through 'ex-da-2e' all drain westerly to wetlands off-site adjacent to the linear trail.

After leaving the site, storm water from Drainage Divide #1 flows northerly and storm water from Drainage Divide #2 flows southerly. While heading in opposite directions, these drainage divides BOTH eventually flow to the Salmon River. Storm water from Drainage Divide #1 flows northerly through the Bear Swamp area via Cattle Lot Brook, to Dickinson Creek in

Marlborough, and ultimately to the Salmon River. Storm water from Drainage Divide #2 flows more directly to the Salmon River via Flat Brook to the south. The Salmon River of course drains to the Connecticut River in Moodus.

The Inland Wetlands and Watercourses Investigation report prepared by James Sipperly, Certified Soil Scientist, is attached as Appendix Sheets A-1 and A-2.

The drainage divides are depicted graphically in Figure 2, an excerpt from the USGS Quad topographic map.

Under post-development conditions, most of the existing sub-watershed areas will be altered. In Drainage Divide #1, proposed drainage area 'PR-DA-1A' is the very northmost part of the site and drains towards Flanders Road. While this sub-watershed will contain a very small portion of the proposed paved roadway, the overall area is drastically reduced to 1.1 acres. The remaining 25.8 acres of Drainage Divide #1 is labeled 'PR-DA-1B' will continue to flow towards the onsite wetlands before leaving the site at the northwest corner. This drainage area will see the largest change in land use cover as it includes approximately 1,300 feet of the proposed paved roadway and includes the developed lots #1 through #6, #11, #14, #15 and part of #12. This sub-watershed is further broken down into the portion that is routed through the proposed detention basin (PR-DA-1B-1) and the smaller portion (PR-DA-1B-2) that is left to flow unimpeded towards the on-site wetlands.

In proposed Drainage Divide #2, PR-DA-2A will see the largest change in land use cover as it includes the last 150 feet of paved roadway, and the developed lots of #7, #9, #13 and part of #12. This sub-watershed is further broken down into the portion that is routed through the proposed detention basin (PR-DA-2A-1) and the smaller portion (PR-DA-2A-2) that is left to flow unimpeded towards the on-site wetlands before leaving the site. The proposed sub-watershed labeled PR-DA-2C contains the oversized rear lot #8 which consists of 11.5 acres. As demonstrated by the calculations, this lot does not necessitate its own storm water attenuation as it is made up for by the effectiveness of Detention Basin #2.

Proposed sub-watersheds 'PR-DA-2C', 'PR-DA-2D' and 'PR-DA-2E 'remain essentially unaltered under post-development conditions.

Based on existing land coverage in the sub-watersheds and the underlying soils, as identified in the official soil survey information on the Web Soil website published by the NRCS, runoff curve numbers (CN) were developed for each sub-watershed area for existing and post-development conditions. Soil Survey data is attached as Appendix Sheets B-1 through B-7. The following runoff coefficients were used based on the hydrologic classification of area soils which include hydrologic class B, C, D and some E soils:

Land Cover	<u>CN</u>
Roofs	98
Pavements	98
Gravel Areas	89
Grass (B)	61
Grass (C)	74
Grass (D/E)	80
Woods (B)	60
Woods (C)	73
Woods (D/E)	79
Residential-2 acres	66
Open Land (B)	67
Open Land (D)	79

The sub-watersheds for pre- and post-development scenarios are depicted on maps WS-1 'Existing Watershed Map' and WS-2 'Proposed Watershed Map' contained at the back of this report. These drainage area maps include existing and proposed topography, depict site coverage features such as pavements and buildings, and also depict the underlying soil types.

The times-of-concentration (Tc) for each sub-watershed were determined for both the pre- and post-development conditions using the SCS Lag Method given parameters of the watershed affecting overland sheet flow, shallow concentrated flow, and channel or pipe flow, where applicable. Storm water hydrographs were developed using SCS TR-55 methodology to ascertain flow rates and volumes, utilizing NOAA 14 published rainfall values for this area within East Hampton. The associated 24-hour rainfall totals utilized are 3.38", 5.18", 6.30", 7.13" and 8.04" for the 2-, 10-, 25-, 50- and 100-year storms, respectively. Rainfall data is attached as Appendix Sheets C-1 and C-2.

Not surprisingly, the developed site is anticipated to increase the peak runoff flow rates from the site. To mitigate this impact, two storm water detention ponds are proposed, one in each drainage divide. Each detention basin will be outfitted with an outlet control structure to meter the discharge flow rates. Each detention basin will provide adequate holding volume for storm water to be retained as it is metered out. The following tables summarize the detention basin stage/ storage relationship:

	Detention 1	Basin 1	Detention Basin 2			
Stage	Elevation	Total	Stage	Elevation	Total	
	(ft)	Storage (cu ft)		(ft)	Storage (cu ft)	
0	436.5	0	0	420.0	0	
0.5	437.0	7,317	1.0	421.0	6,774	
2.5	438.0	23,736	2.0	422.0	16,453	
2.5	439.0	42,615	3.0	423.0	28,504	
3.5	440.0	64,038	4.0	424.0	42,365	
4.5	441.0	88,182	5.0	425.0	58,106	
5.5	442.0	115,219	6.0	426,0	75,802	

The following table summarizes the overall site runoff at the two analysis points for pre-, post-developed, and post-developed with storm water mitigation (detention basins) for the design storms:

Storm Pre-Dev		veloped		veloped	Post-Developed		
			(no Det	tention)	(w/ Detention)		
	Drainage	Drainage	Drainage	Drainage	Drainage	Drainage	
	Divide 1	Divide 2	Divide 1	Divide 2	Divide 1	Divide 2	
2-year	13.7 cfs	27.1 cfs	18.6 cfs	30.0 cfs	11.5 cfs	25.2 cfs	
10-year	30.8 cfs	56.7 cfs	39.7 cfs	60.8 cfs	28.2 cfs	50.5 cfs	
25-year	42.4 cfs	76.2 cfs	53.8 cfs	81.0 cfs	36.9 cfs	66.8 cfs	
50-year			64.4 cfs	96.1 cfs	42.8 cfs	79.1 cfs	
100-year	61.1 cfs	107.1 cfs	76.2 cfs	112.8 cfs	48.5 cfs	93.2 cfs	

As summarized above and in the calculations contained in the Appendix, with provision of the proposed storm water detention basins, the post-development peak runoff flows will be effectively reduced to BELOW pre-development levels in both Drainage Divides for all storm events.

Considering the 100-year storm, pre-development flows from Drainage Divide #1 will be reduced from 61.1 cfs to 48.5 cfs considering the effectiveness of Detention Basin #1. Similarly, pre-development flows from Drainage Divide #2 will be reduced from 107.1 cfs to 93.2 cfs considering the effectiveness of Detention Basin #2.

Drainage calculations are attached in the Appendix as sheets D-1 through D-50 and existing and proposed drainage area maps are attached as Maps WS-1 and WS-2.

#### **APPENDIX**

Figure 1

Site Vicinity

Figure 2

**USGS Quadrangle Topo Excerpt** 

A-1 thru A-2

Soil Scientist Report

B-1 thru B-7

NRCS (SCS) Soil Data

C-1 thru C-

**NOAA** Precipitation Frequency Data

D-1 thru D-50

**Drainage Calculations** 

Watershed Schematic

Peak Flow Summary (All Storms) 2-Year Storm Hydrographs Summary 10-Year Storm Hydrographs Summary 25-Year Storm Hydrographs Summary 50-Year Storm Hydrographs Summary 100-Year Storm Hydrographs Summary

**Existing Condition Hydrographs** 

Tc Worksheets

Proposed Condition Hydrographs

Tc Worksheets

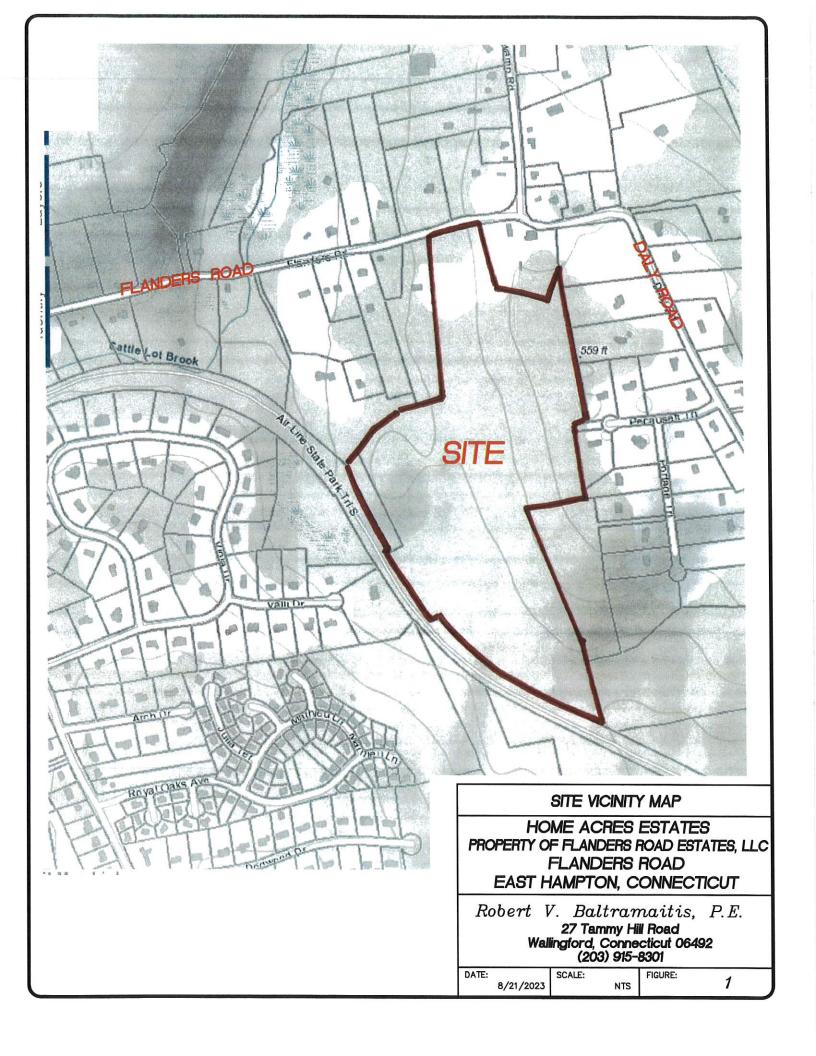
Post-Dev (No Detention) Hydrograph Combo Detention System Hydrographs & Pond Data Post-Dev (With Detention) Hydrograph Combo

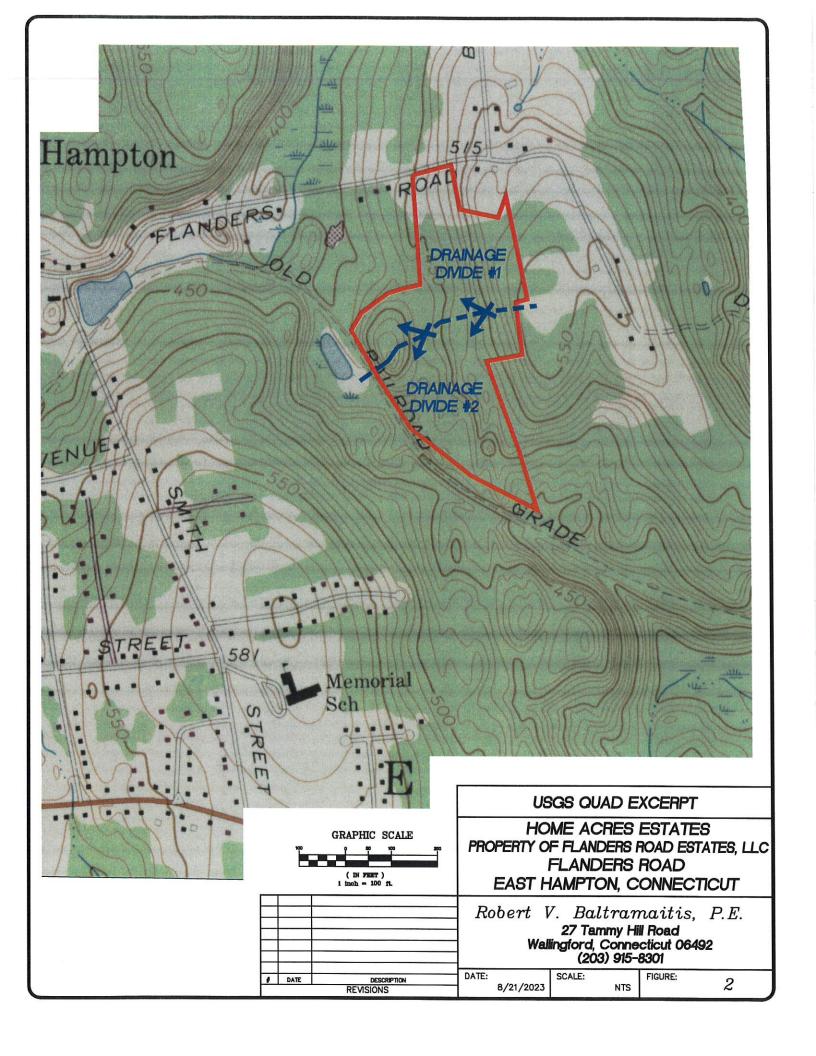
Map WS-1

**Existing Watershed Map** 

Map WS-2

**Proposed Watershed Map** 





#### JAMES SIPPERLY

# CERTIFIED SOIL SCIENTIST CONNECTICUT WETLAND SCIENTIST 401 SALEM TURNPIKE BOZRAH, CT 06334 860-334-7073

james.sipperly.js@gmail.com

Joshua Wilson, Chairman
Inland Wetlands and Watercourses Commission
Town of East Hampton, CT
East Hampton Town Hall
1 Community Drive
East Hampton, CT 06424

July 22, 2023

RE: INLAND WETLANDS AND WATERCOURSES INVESTIGATION AND DELINEATION- FLANDERS ROAD ESTATES LLC, MAP 26 BLOCK 87 LOT 6

Dear Chairman Wilson:

On October 22, 2023 I conducted a field site visitation to the property referenced above to delineate any inland wetland soils and/or watercourses that may exist on the site.

I sampled the soils throughout the site using a soil auger to a depth of 2-3 feet. Based on my field observations and using the guidelines established by the National Cooperative Soil Survey and as defined by the Connecticut General Statutes, I delineated the inland wetland soils and watercourses on the site.

I delineated the inland wetland soils using blue flagging numbered 1-32, 1A-53A, 1B-9B respectively.

The inland wetland soils delineated by flags 1-32 are classified as a very poorly drained Saco silt loam. These soils are formed in alluvial stratified sand and silt loams. These soils are on low flood plains along streams and rivers and are frequently flooded. This area is adjacent to the existing airline trail and has had some grading and deposition from the work along the trail for drainage improvements.

For the inland wetland flag series 1A-53A, only flags 14A-49A are shown on the plans because when I conducted my soil investigation the property lines were not clear and the adjacent property removed the other flags that were advertently placed on his property. We spoke in the field to resolve that issue. These wetland soils are classified as a Ridgebury, Leicester Whitman extremely stony fine sandy loam. These soils are mapped together as a complex because of their similar physical characteristics, use ad management. They are found in drainage ways in glacial till uplands.

Inland wetland flags 1B-9B are also classified as a Ridgebury, Leicester Whitman fine sandy loam located in an isolated depression in the glacial till uplands.

The upland soils are classified as a moderately well drained Woodbridge fine sandy loam and transition into a well drained Paxton fine sandy loam and well drained Canton and Charlton fine sandy loam.

All of the wetland areas located on the site are classified as a forested wetland general classification. This wetland area has functions that include: groundwater recharge and discharge, sediment stabilization, nutrient removal and transformation, product export, and wildlife diversity and habitat. The vegetative over-story includes maples, ash, black cherry and poplar. Shrub species include, winterberry, spice bush, silky dogwood. The herbaceous layer includes sensitive fern, poison ivy, wild grape, jack in the pulpit and skunk cabbage.

The wetland area has a diverse native vegetation population at the herbaceous, shrub and tree layers. No evidence of invasive species was observed. I expect that an abundance of wildlife utilizes this wetland corridor as a source of food, water and shelter.

The proposed development in the upland review area will not be disturbing any wetlands and/or watercourses on or adjacent to the site. For that reason, the inland wetlands will continue to perform their functions as they currently do.

With any proposed project a comprehensive erosion and sedimentation control plan well designed and properly installed and maintained is the key to a successful project. Regular inspections should occur, especially after storm events of more than 1.5" of rain.

After reviewing the erosion and sedimentation control plans and the storm water design features, it is my professional opinion that the proposed construction activities will not have a significant adverse effect on the adjacent inland wetlands and/or watercourse on or off the site.

If you have any questions or require additional information, please contact me at the telephone number referenced above.

Very truly yours,

James Sipperly

James Sipperly
Certified Soil Scientist, Society of Soil Scientists
Connecticut Wetland Scientist, Connecticut Association of Wetland Scientists



## **Map Unit Legend**

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI		
3	Ridgebury, Leicester, and Whitman soils, 0 to 8 percent slopes, extremely stony	14.3	3.6%		
17	Timakwa and Natchaug soils, 0 to 2 percent slopes	12.5	3.2%		
36B	Windsor loamy sand, 3 to 8 percent slopes	7.3	1.9%		
45B	Woodbridge fine sandy loam, 3 to 8 percent slopes	13.3	3.4%		
45C	Woodbridge fine sandy loam, 8 to 15 percent slopes	6.6	1.7%		
46B > Wx	Woodbridge fine sandy loam, 0 to 8 percent slopes, very stony	11.5	2.9%		
46C	Woodbridge fine sandy loam, 8 to 15 percent slopes, very stony	24.1			
60C	Canton and Charlton fine sandy loams, 8 to 15 percent slopes	2.2	0.6%		
60D	Canton and Charlton soils, 15 to 25 percent slopes	4.3	1.1%		
61B	Canton and Charlton fine sandy loams, 0 to 8 percent slopes, very stony	1.0	0.2%		
61C	Canton and Charlton fine sandy loams, 8 to 15 percent slopes, very stony	4.8	1.2%		
62C	Canton and Charlton fine sandy loams, 3 to 15 percent slopes, extremely stony	2.5	0.6%		
62D Ca	Canton and Charlton fine sandy loams, 15 to 35 percent slopes, extremely stony	12.0	3.1%		
71E	Nipmuck-Brimfield-Rock outcrop complex, 15 to 45 percent slopes	51.2	13.0%		
72C Ni	Nipmuck-Brookfield complex, 3 to 15 percent slopes, very rocky	22.5	5.79		
72E	Nipmuck-Brookfield complex, 15 to 45 percent slopes, very rocky	127,4	32.4%		
73C	Charlton-Chatfield complex, 0 to 15 percent slopes, very rocky	5.9	1.59		

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI	
84B	Paxton and Montauk fine sandy loams, 3 to 8 percent slopes	17.0	4.3%	
84C	Paxton and Montauk fine sandy loams, 8 to 15 percent slopes	8.9	2.3%	
84D	Paxton and Montauk fine sandy loams, 15 to 25 percent slopes	3.4	2.5%	
85B > Pa	Paxton and Montauk fine sandy loams, 3 to 8 percent slopes, very stony	9.7		
85C	Paxton and Montauk fine sandy loams, 8 to 15 percent slopes, very stony	14.8	3.8%	
86D	Paxton and Montauk fine sandy loams, 15 to 35 percent slopes, extremely stony	14.6	3.7%	
W	Water	1.3	0.3%	
Totals for Area of Interest	•	393.1	100.0%	



#### NOAA Atlas 14, Volume 10, Version 3 Location name: Town of East Hampton, Connecticut, USA\*

Latitude: 41.5811°, Longitude: -72.4811°

Elevation: 490 ft\*\*

\* source: ESRI Maps

\*\* source: USGS



#### POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sandra Pavlovic, Michael St. Laurent, Carl Trypaluk, Dale Unruh, Orlan Wilhite

NOAA, National Weather Service, Silver Spring, Maryland

PF tabular | PF graphical | Maps & aerials

#### PF tabular

PDS-I	PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches) <sup>1</sup>												
Duration				Average	recurrence	interval (y	ears)						
Duration	1	2	5	10	25	50	100	200	500	1000			
5-min	<b>0.334</b> (0.259-0.421)	<b>0.405</b> (0.314-0.511)	<b>0.521</b> (0.402-0.659)	<b>0.617</b> (0.474-0.784)	<b>0.750</b> (0.558-0.994)	<b>0.850</b> (0.619-1.15)	<b>0.954</b> (0.677-1.34)	<b>1.07</b> (0.720-1.53)	<b>1.24</b> (0.803-1.83)	1.38 (0.872-2.07)			
10-min	<b>0.474</b> (0.367-0.596)	<b>0.574</b> (0.445-0.724)	<b>0.738</b> (0.569-0.933)	<b>0.874</b> (0.670-1.11)	<b>1.06</b> (0.790-1.41)	<b>1.20</b> (0.878-1.63)	<b>1.35</b> (0.958-1.89)	<b>1.52</b> (1.02-2.17)	<b>1.76</b> (1.14-2.59)	1.95 (1.24-2.94)			
15-min	<b>0.557</b> (0.432-0.702)	<b>0.676</b> (0.523-0.851)	<b>0.870</b> (0.670-1.10)	<b>1.03</b> (0.791-1.31)	<b>1.25</b> (0.930-1.66)	<b>1.42</b> (1.03-1.92)	<b>1.59</b> (1.13-2.23)	<b>1.79</b> (1.20-2.55)	<b>2.07</b> (1.34-3.05)	<b>2.30</b> (1.45-3.46)			
30-min	<b>0.761</b> (0.590-0.958)	<b>0.923</b> (0.714-1.16)	<b>1.19</b> (0.916-1.50)	<b>1.40</b> (1.08-1.78)	<b>1.71</b> (1.27-2.26)	<b>1.93</b> (1.41-2.61)	<b>2.17</b> (1.54-3.04)	<b>2.44</b> (1.64-3.48)	<b>2.82</b> (1.83-4.16)	<b>3.13</b> (1.98-4.71)			
60-min	<b>0.965</b> (0.748-1.22)	<b>1.17</b> (0.905-1.47)	<b>1.50</b> (1.16-1.90)	<b>1.78</b> (1.37-2.26)	<b>2.16</b> (1.61-2.86)	<b>2.45</b> (1.78-3.31)	<b>2.75</b> (1.95-3.85)	3.09 (2.08-4.41)	3.57 (2.31-5.27)	3.97 (2.51-5.97)			
2-hr	<b>1.27</b> (0.994-1.59)	<b>1.53</b> (1.19-1.91)	<b>1.95</b> (1.52-2.45)	<b>2.30</b> (1.78-2.90)	<b>2.78</b> (2.08-3.66)	<b>3.14</b> (2.31-4.22)	<b>3.52</b> (2.52-4.91)	3.96 (2.68-5.62)	<b>4.62</b> (3.00-6.76)	<b>5.17</b> (3.28-7.71)			
3-hr	<b>1.48</b> (1.16-1.85)	<b>1.78</b> (1.40-2.22)	<b>2.26</b> (1.77-2.83)	<b>2.66</b> (2.07-3.34)	<b>3.21</b> (2.42-4.22)	<b>3.62</b> (2.68-4.86)	<b>4.06</b> (2.92-5.65)	<b>4.58</b> (3.10-6.46)	<b>5.36</b> (3.49-7.80)	<b>6.01</b> (3.82-8.92)			
6-hr	<b>1.90</b> (1.50-2.35)	<b>2.27</b> (1.80-2.82)	<b>2.89</b> (2.28-3.59)	<b>3.40</b> (2.66-4.24)	<b>4.10</b> (3.11-5.35)	<b>4.62</b> (3.44-6.16)	<b>5.18</b> (3.75-7.17)	<b>5.85</b> (3.98-8.19)	<b>6.86</b> (4.48-9.91)	<b>7.72</b> (4.92-11.4)			
12-hr	<b>2.36</b> (1.88-2.90)	<b>2.84</b> (2.26-3.49)	3.63 (2.88-4.47)	<b>4.28</b> (3.38-5.30)	<b>5.17</b> (3.96-6.70)	<b>5.84</b> (4.38-7.72)	<b>6.56</b> (4.78-9.00)	<b>7.41</b> (5.06-10.3)	<b>8.70</b> (5.70-12.5)	9.79 (6.27-14.3)			
24-hr	<b>2.78</b> (2.24-3.39)	3.38 (2.72-4.13)	<b>4.36</b> (3.50-5.34)	<b>5.18</b> (4.12-6.38)	<b>6.30</b> (4.86-8.12)	<b>7.13</b> (5.39-9.38)	<b>8.04</b> (5.91-11.0)	<b>9.13</b> (6.26-12.6)	<b>10.8</b> (7.11-15.4)	<b>12.2</b> (7.86-17.7)			
2-day	<b>3.12</b> (2.53-3.78)	<b>3.85</b> (3.12-4.66)	<b>5.04</b> (4.07-6.13)	<b>6.03</b> (4.84-7.36)	<b>7.39</b> (5.74-9.47)	<b>8.38</b> (6.39-11.0)	<b>9.48</b> (7.05-13.0)	<b>10.9</b> (7.48-14.9)	13.0 (8.60-18.4)	14.9 (9.62-21.4)			
3-day	<b>3.38</b> (2.76-4.08)	<b>4.18</b> (3.41-5.05)	<b>5.49</b> (4.46-6.65)	<b>6.57</b> (5.30-8.00)	<b>8.06</b> (6.30-10.3)	<b>9.15</b> (7.01-12.0)	<b>10.4</b> (7.73-14.1)	<b>11.9</b> (8.20-16.2)	<b>14.3</b> (9.46-20.1)	<b>16.4</b> (10.6-23.4)			
4-day	<b>3.63</b> (2.97-4.37)	<b>4.48</b> (3.66-5.39)	<b>5.86</b> (4.78-7.08)	<b>7.02</b> (5.68-8.51)	<b>8.60</b> (6.74-10.9)	<b>9.76</b> (7.50-12.7)	<b>11.0</b> (8.26-15.0)	<b>12.7</b> (8.75-17.2)	<b>15.2</b> (10.1-21.3)	17.5 (11.3-24.8)			
7-day	<b>4.31</b> (3.56-5.16)	<b>5.26</b> (4.33-6.30)	<b>6.81</b> (5.59-8.18)	<b>8.10</b> (6.60-9.77)	<b>9.87</b> (7.78-12.5)	<b>11.2</b> (8.62-14.4)	<b>12.6</b> (9.45-16.9)	<b>14.4</b> (9.98-19.4)	<b>17.2</b> (11.4-23.8)	<b>19.6</b> (12.7-27.7)			
10-day	<b>5.00</b> (4.14-5.96)	<b>6.01</b> (4.97-7.16)	<b>7.65</b> (6.30-9.15)	<b>9.01</b> (7.37-10.8)	<b>10.9</b> (8.60-13.7)	<b>12.3</b> (9.48-15.7)	<b>13.8</b> (10.3-18.3)	<b>15.6</b> (10.9-20.9)	<b>18.5</b> (12.3-25.5)	<b>20.9</b> (13.6-29.3)			
20-day	<b>7.17</b> (5.99-8.48)	<b>8.25</b> (6.88-9.77)	<b>10.0</b> (8.32-11.9)	<b>11.5</b> (9.48-13.7)	<b>13.5</b> (10.7-16.7)	<b>15.0</b> (11.6-18.9)	<b>16.6</b> (12.4-21.6)	<b>18.4</b> (12.9-24.4)	<b>21.0</b> (14.1-28.7)	<b>23.1</b> (15.0-32.2)			
30-day	<b>9.00</b> (7.56-10.6)	<b>10.1</b> (8.49-11.9)	<b>11.9</b> (9.98-14.1)	<b>13.5</b> (11.2-16.0)	<b>15.6</b> (12.4-19.1)	<b>17.1</b> (13.3-21.4)	18.8 (14.0-24.1)	<b>20.5</b> (14.4-27.0)	<b>22.8</b> (15.3-31.0)	<b>24.6</b> (16.1-34.1)			
45-day	<b>11.3</b> (9.53-13.2)	<b>12.5</b> (10.5-14.6)	<b>14.3</b> (12.0-16.9)	<b>15.9</b> (13.3-18.8)	<b>18.1</b> (14.5-22.0)	<b>19.8</b> (15.4-24.5)	<b>21.4</b> (15.9-27.2)	23.0 (16.3-30.2)	<b>25.1</b> (16.9-33.9)	<b>26.6</b> (17.4-36.6)			
60-day	<b>13.2</b> (11.2-15.4)	<b>14.4</b> (12.2-16.9)	<b>16.4</b> (13.8-19.2)	<b>18.0</b> (15.1-21.2)	<b>20.2</b> (16.2-24.5)	<b>22.0</b> (17.1-27.0)	<b>23.7</b> (17.6-29.8)	<b>25.2</b> (17.9-32.9)	<b>27.1</b> (18.3-36.4)	<b>28.4</b> (18.5-38.9)			

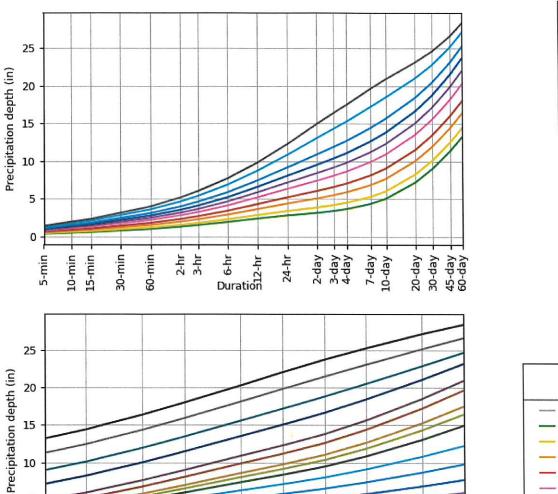
Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values. Please refer to NOAA Atlas 14 document for more information.

Back to Top

PF graphical

#### PDS-based depth-duration-frequency (DDF) curves Latitude: 41.5811°, Longitude: -72.4811°



Duration 5-min 2-day 3-day 10-min 15-min 4-day 30-min 7-day 60-min 10-day 2-hr 20-day 3-hr 30-day 6-hr 45-day 12-hr - 60-day 24-hr

Average recurrence interval (years)

> 2 5 10

> 25 50

100 200 500

- 1000

NOAA Atlas 14, Volume 10, Version 3

5

10

25

Average recurrence interval (years)

50

10

5

0

Created (GMT): Fri Aug 18 20:28:12 2023

1000

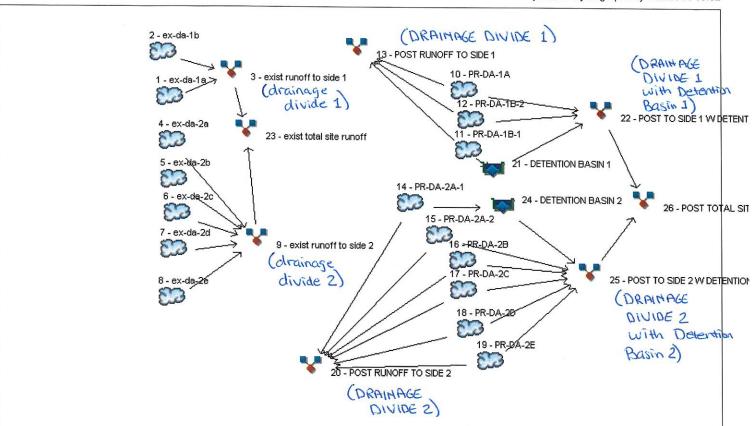
500

Back to Top

200

#### Maps & aerials

Small scale terrain



L	ea	e	n	d

Hyd.	<u>Origin</u>	Description
1	SCS Runoff	ex-da-1a
2	SCS Runoff	ex-da-1b
3	Combine	exist runoff to side 1
4	SCS Runoff	ex-da-2a
5	SCS Runoff	ex-da-2b
6	SCS Runoff	ex-da-2c
7	SCS Runoff	ex-da-2d
8	SCS Runoff	ex-da-2e
9	Combine	exist runoff to side 2
10	SCS Runoff	PR-DA-1A
11	SCS Runoff	PR-DA-1B-1
12	SCS Runoff	
13	Combine	POST RUNOFF TO SIDE 1 (no detention)
14	SCS Runoff	PR-DA-2A-1
15	SCS Runoff	PR-DA-2A-2
16	SCS Runoff	PR-DA-2B
17	SCS Runoff	PR-DA-2C
18	SCS Runoff	PR-DA-2D
19	SCS Runoff	PR-DA-2E
20	Combine	POST RUNOFF TO SIDE 2 (no detention)
21	Reservoir	DETENTION BASIN 1
22	Combine	POST TO SIDE 1 W DETENTION
23	Combine	exist total site runoff
24	Reservoir	DETENTION BASIN 2
25	Combine	POST TO SIDE 2 W DETENTION
26	Combine	POST TOTAL SITE RUNOFF WIDETENTION

Project: flanders storm water.gpw

Monday, Aug 21, 2023

## Hydrograph Return Period Recap

Hydraflow Hydrographs by Intelisolve v9.02

1 SCS Run 2 SCS Run 3 Combine 4 SCS Run 5 SCS Run 6 SCS Run 7 SCS Run 8 SCS Run 9 Combine 10 SCS Run 11 SCS Run 12 SCS Run 13 Combine 14 SCS Run 15 SCS Run 16 SCS Run 17 SCS Run 18 SCS Run 19 SCS Run 19 SCS Run 19 SCS Run 10 SCS Run 11 SCS Run 12 SCS Run 13 Combine 14 SCS Run 15 SCS Run 16 SCS Run 17 SCS Run 18 SCS Run 19 SCS Run 19 SCS Run	noff		2-Yr  5.871 12.56 13.67 11.25 2.498 11.89 1.769 2.515 27.15 1.317 15.30 4.403 18.57 9.811	3-Yr	5-Yr	10-Yr  11.95 28.46 30.79 24.36 5.083 24.26 3.600 5.102 56.68 2.672 33.13	25-Yr  15.90 39.27 42.40 33.11 6.766 32.33 4.798 6.794 76.19 3.559 45.02	18.86 47.50 51.25 39.76 8.028 38.38 5.696 8.070 90.88 4.227	22.12 56.67 61.09 47.14 9.418 45.05 6.685 9.476 107.13 4.964	ex-da-1a ex-da-1b exist runoff to side 1 ex-da-2a ex-da-2b ex-da-2c ex-da-2d ex-da-2d ex-da-2e exist runoff to side 2 PR-DA-1A
2 SCS Run 3 Combine 4 SCS Run 5 SCS Run 6 SCS Run 7 SCS Run 8 SCS Run 9 Combine 10 SCS Run 11 SCS Run 12 SCS Run 13 Combine 14 SCS Run 15 SCS Run 16 SCS Run 17 SCS Run 18 SCS Run 19 SCS Run 19 SCS Run	noff  1, 2  noff  noff  noff  noff  4, 5, 6, 7  noff  noff  noff  noff  noff  noff  noff  noff	, 8	12.56 13.67 11.25 2.498 11.89 1.769 2.515 27.15 1.317 15.30 4.403 18.57			28.46 30.79 24.36 5.083 24.26 3.600 5.102 56.68 2.672 33.13	39.27 42.40 33.11 6.766 32.33 4.798 6.794 76.19 3.559	47.50 51.25 39.76 8.028 38.38 5.696 8.070 90.88 4.227	56.67 61.09 47.14 9.418 45.05 6.685 9.476 107.13	ex-da-1b  exist runoff to side 1  ex-da-2a  ex-da-2b  ex-da-2c  ex-da-2d  ex-da-2e  exist runoff to side 2
3 Combine 4 SCS Run 5 SCS Run 6 SCS Run 7 SCS Run 8 SCS Run 9 Combine 10 SCS Run 11 SCS Run 12 SCS Run 13 Combine 14 SCS Run 15 SCS Run 16 SCS Run 17 SCS Run 17 SCS Run 18 SCS Run 19 SCS Run	1, 2 noff	, 8	13.67 11.25 2.498 11.89 1.769 2.515 27.15 1.317 15.30 4.403 18.57			30.79 24.36 5.083 24.26 3.600 5.102 56.68 2.672 33.13	42.40 33.11 6.766 32.33 4.798 6.794 76.19 3.559	51.25 39.76 8.028 38.38 5.696 8.070 90.88 4.227	61.09 47.14 9.418 45.05 6.685 9.476 107.13	exist runoff to side 1 ex-da-2a ex-da-2b ex-da-2c ex-da-2d ex-da-2d ex-da-2e
4 SCS Run 5 SCS Run 6 SCS Run 7 SCS Run 8 SCS Run 9 Combine 10 SCS Run 11 SCS Run 12 SCS Run 13 Combine 14 SCS Run 15 SCS Run 16 SCS Run 17 SCS Run 18 SCS Run 19 SCS Run	noff	, 8	11.25 2.498 11.89 1.769 2.515 27.15 1.317 15.30 4.403 18.57			24.36 5.083 24.26 3.600 5.102 56.68 2.672 33.13	33.11 6.766 32.33 4.798 6.794 76.19 3.559	39.76 8.028 38.38 5.696 8.070 90.88 4.227	47.14 9.418 45.05 6.685 9.476 107.13	ex-da-2a ex-da-2b ex-da-2c ex-da-2d ex-da-2e exist runoff to side 2
5 SCS Run 6 SCS Run 7 SCS Run 8 SCS Run 9 Combine 10 SCS Run 11 SCS Run 12 SCS Run 13 Combine 14 SCS Run 15 SCS Run 16 SCS Run 17 SCS Run 18 SCS Run 19 SCS Run	noff	, 8	2.498 11.89 1.769 2.515 27.15 1.317 15.30 4.403 18.57			5.083 24.26 3.600 5.102 56.68 2.672 33.13	6.766 32.33 4.798 6.794 76.19 3.559	8.028 38.38 5.696 8.070 90.88 4.227	9.418 45.05 6.685 9.476 107.13	ex-da-2b ex-da-2c ex-da-2d ex-da-2e exist runoff to side 2
6 SCS Run 7 SCS Run 8 SCS Run 9 Combine 10 SCS Run 11 SCS Run 12 SCS Run 13 Combine 14 SCS Run 15 SCS Run 16 SCS Run 17 SCS Run 18 SCS Run 19 SCS Run	noff	, 8	11.89 1.769 2.515 27.15 1.317 15.30 4.403 18.57			24.26 3.600 5.102 56.68 2.672 33.13	32.33 4.798 6.794 76.19 3.559	38.38 5.696 8.070 90.88 4.227	45.05 6.685 9.476 107.13	ex-da-2c ex-da-2d ex-da-2e exist runoff to side 2
7 SCS Run 8 SCS Run 9 Combine 10 SCS Run 11 SCS Run 12 SCS Run 13 Combine 14 SCS Run 15 SCS Run 16 SCS Run 17 SCS Run 18 SCS Run 19 SCS Run	noff 4, 5, 6, 7 noff noff noff noff noff noff	, 8	1.769 2.515 27.15 1.317 15.30 4.403 18.57			3.600 5.102 56.68 2.672 33.13	4.798 6.794 76.19 3.559	5.696 8.070 90.88 4.227	6.685 9.476 107.13	ex-da-2d ex-da-2e exist runoff to side 2
8 SCS Run 9 Combine 10 SCS Run 11 SCS Run 12 SCS Run 13 Combine 14 SCS Run 15 SCS Run 16 SCS Run 17 SCS Run 18 SCS Run 19 SCS Run	4, 5, 6, 7 noff noff 10, 11, 1 noff	, 8	2.515 27.15 1.317 15.30 4.403 18.57			5.102 56.68 2.672 33.13	6.794 76.19 3.559	8.070 90.88 4.227	9.476 107.13	ex-da-2e exist runoff to side 2
9 Combine 10 SCS Run 11 SCS Run 12 SCS Run 13 Combine 14 SCS Run 15 SCS Run 16 SCS Run 17 SCS Run 18 SCS Run 19 SCS Run	4, 5, 6, 7 noff noff 10, 11, 1 noff	, 8	27.15 1.317 15.30 4.403 18.57			56.68 2.672 33.13	76.19 3.559	90.88 4.227	107.13	exist runoff to side 2
10 SCS Run 11 SCS Run 12 SCS Run 13 Combine 14 SCS Run 15 SCS Run 16 SCS Run 17 SCS Run 18 SCS Run 19 SCS Run	noff noff noff noff noff	2	1.317 15.30 4.403 18.57			2.672 33.13	3.559	4.227	22 422002	
11 SCS Run 12 SCS Run 13 Combine 14 SCS Run 15 SCS Run 16 SCS Run 17 SCS Run 18 SCS Run 19 SCS Run	noff noff noff noff	2	15.30 4.403 18.57			33.13			4.964	PR-DA-1A
12 SCS Run 13 Combine 14 SCS Run 15 SCS Run 16 SCS Run 17 SCS Run 18 SCS Run 19 SCS Run	noff noff	2	4.403 18.57			4000/2002/02/02	45.02	1/20/02/20		2
13 Combine 14 SCS Run 15 SCS Run 16 SCS Run 17 SCS Run 18 SCS Run 19 SCS Run	10, 11, 1 noff	2	18.57			0.700		54.06	64.10	PR-DA-1B-1
<ul> <li>SCS Run</li> </ul>	noff		100.200.000.000.000		1 2200000000	8.763	11.61	13.74	16.09	PR-DA-1B-2
<ul> <li>15 SCS Run</li> <li>16 SCS Run</li> <li>17 SCS Run</li> <li>18 SCS Run</li> <li>19 SCS Run</li> </ul>	noff		9.811			39.69	53.76	64.42	76.22	POST RUNOFF TO SIDE 1
16 SCS Rur 17 SCS Rur 18 SCS Rur 19 SCS Rur			ı			19.62	26.02	30.82	36.10	PR-DA-2A-1
17 SCS Run 18 SCS Run 19 SCS Run	noff		4.325			8.802	11.72	13.91	16.31	PR-DA-2A-2
18 SCS Rur 19 SCS Rur	ı		2.498			5.083	6.766	8.028	9.418	PR-DA-2B
19 SCS Rur	noff		11.59			23.64	31.51	37.40	43.90	PR-DA-2C
	noff		1.769			3.600	4.798	5.696	6.685	PR-DA-2D
00 0 11	noff		2.515			5.102	6.794	8.070	9.476	PR-DA-2E
20 Combine	14, 15, 1	6, 17 <del>, 18,</del> 1	9 30.01			60.83	80.99	96.11	112.76	POST RUNOFF TO SIDE 2
21 Reservoi	ir 11		10.37			25.67	33.70	39.18	44.50	DETENTION BASIN 1
22 Combine	10, 12, 2	1	11.51			28.20	36.92	42.79	48.52	POST TO SIDE 1 W DETENTION
23 Combine	3, 9,		39.79			85.20	115.40	138.25	163.56	exist total site runoff
24 Reservoi	ir 14		5.950			11.78	16.88	20.97	25.37	DETENTION BASIN 2
25 Combine	15, 16, 1	7, 18 <del>, 19,</del> 2	4 25.20			50.46	66.82	79.13	93.18	POST TO SIDE 2 W DETENTION
26 Combine	22, 25		33.21			72.17	97.98	117.87	139.39	POST TOTAL SITE RUNOFF

Proj. file: flanders storm water.gpw

Tuesday, Aug 22, 2023

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time Interval (min)	Time to peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph description
1	SCS Runoff	5.871	2	726	20,323				ex-da-1a
2	SCS Runoff	12.56	2	760	93,610				ex-da-1b
3	Combine	13.67	2	754	113,934	1, 2			exist runoff to side 1
4	SCS Runoff	11.25	2	754	75,458				ex-da-2a
5	SCS Runoff	2.498	2	736	11,976			******	ex-da-2b
6	SCS Runoff	11.89	2	742	63,109				ex-da-2c
7	SCS Runoff	1.769	2	732	7,300		Wateran		ex-da-2d
8	SCS Runoff	2.515	2	734	11,231		W		ex-da-2e
9	Combine	27.15	2	744	169,076	4, 5, 6, 7, 8			exist runoff to side 2
10	SCS Runoff	1.317	2	734	5,883				PR-DA-1A
11	SCS Runoff	15.30	2	754	102,604				PR-DA-1B-1
12	SCS Runoff	4.403	2	734	19,587		~~~~	<b></b>	PR-DA-1B-2
13	Combine	18.57	2	748	128,075	10, 11, 12	00078F		POST RUNOFF TO SIDE 1
14	SCS Runoff	9.811	2	752	64,339				PR-DA-2A-1
15	SCS Runoff	4.325	2	738	21,585	M 74 MAN			PR-DA-2A-2
16	SCS Runoff	2.498	2	736	11,976				PR-DA-2B
17	SCS Runoff	11.59	2	742	61,505			праны	PR-DA-2C
18	SCS Runoff	1.769	2	732	7,300				PR-DA-2D
19	SCS Runoff	2.515	2	734	11,231		P W TER HELL		PR-DA-2E
20	Combine	30.01	2	742	177,937	14, 15, 16,	17, 18 <del>, 1</del> 9		POST RUNOFF TO SIDE 2
21	Reservoir	10.37	2	774	70,036	11	437,82	20,778	DETENTION BASIN 1
22	Combine	11.51	2	772	95,507	10, 12, 21			POST TO SIDE 1 W DETENTION
23	Combine	39.79	2	748	283,009	3, 9,	73770 F		exist total site runoff
24	Reservoir	5.950	2	776	51,932	14	421.68	13,298	DETENTION BASIN 2
25	Combine	25.20	2	740	165,530	15, 16, 17,	18, 19 <sub>;-</sub> 24		POST TO SIDE 2 W DETENTION
26	Combine	33.21	2	742	261,037	22, 25		Pi finis shidos	POST TOTAL SITE RUNOFF
flan	ders storm w	/ater.gpw			Return F	Period: 2 Ye	ear	Tuesday,	Aug 22, 2023

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to peak (min)	Hyd. volume (cuft)	inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph description
1	SCS Runoff	11.95	2	726	40,792				ex-da-1a
2	SCS Runoff	28.46	2	756	202,824				ex-da-1b
3	Combine	30.79	2	754	243,616	1, 2			exist runoff to side 1
4	SCS Runoff	24.36	2	752	158,391				ex-da-2a
5	SCS Runoff	5,083	2	736	24,038				ex-da-2b
6	SCS Runoff	24,26	2	740	126,670				ex-da-2c
7	SCS Runoff	3,600	2	730	14,653				ex-da-2d
8	SCS Runoff	5.102	2	734	22,543		W-1		ex-da-2e
9	Combine	56.68	2	742	346,295	4, 5, 6, 7, 8			exist runoff to side 2
10	SCS Runoff	2.672	2	734	11,808				PR-DA-1A
11	SCS Runoff	33.13	2	752	215,373				PR-DA-1B-1
12	SCS Runoff	8,763	2	734	38,756		754545		PR-DA-1B-2
13	Combine	39.69	2	748	265,937	10, 11, 12			POST RUNOFF TO SIDE 1
14	SCS Runoff	19.62	2	750	127,303				PR-DA-2A-1
15	SCS Runoff	8.802	2	738	43,325				PR-DA-2A-2
16	SCS Runoff	5.083	2	736	24,038				PR-DA-2B
17	SCS Runoff	23.64	2	740	123,449				PR-DA-2C
18	SCS Runoff	3.600	2	730	14,653		11 P.V. 1754 P.		PR-DA-2D
19	SCS Runoff	5.102	2	734	22,543	2722	*****		PR-DA-2E
20	Combine	60.83	2	740	355,310	14, 15, 16,	17, 1 <del>8, 19</del>		POST RUNOFF TO SIDE 2
21	Reservoir	25.67	2	768	169,860	11	438.75	37,934	DETENTION BASIN 1
22	Combine	28.20	2	766	220,424	10, 12, 21		20000	POST TO SIDE 1 W DETENTION
23	Combine	85.20	2	746	589,911	3, 9,	********		exist total site runoff
24	Reservoir	11.78	2	776	108,655	14	423.03	28,860	DETENTION BASIN 2
25	Combine	50.46	2	738	336,663	15, 16, 17,	18, 19 <del>, 24</del>		POST TO SIDE 2 W DETENTION
26	Combine	72.17	2	742	557,086	22, 25			POST TOTAL SITE RUNOFF
flar	nders storm w	/ater.gpw	1		Return F	Period: 10	Year	Tuesday,	Aug 22, 2023

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time Interval (min)	Time to peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph description
1	SCS Runoff	15.90	2	726	54,468			********	ex-da-1a
2	SCS Runoff	39.27	2	756	278,022			******	ex-da-1b
3	Combine	42.40	2	752	332,490	1,2	~~~~~	44 M 30 M boss	exist runoff to side 1
4	SCS Runoff	33.11	2	752	214,783			<u></u>	ex-da-2a
5	SCS Runoff	6,766	2	736	32,097		Selection on		ex-da-2b
6	SCS Runoff	32.33	2	740	169,138				ex-da-2c
7	SCS Runoff	4.798	2	730	19,566				ex-da-2d
В	SCS Runoff	6.794	2	732	30,101			B-GMMB-G	ex-da-2e
9	Combine	76.19	2	742	465,685	4, 5, 6, 7, 8	21 27 W 25 25 46		exist runoff to side 2
10	SCS Runoff	3.559	2	732	15,767		******		PR-DA-1A
11	SCS Runoff	45.02	2	752	292,053				PR-DA-1B-1
12	SCS Runoff	11.61	2	732	51,494			-07577	PR-DA-1B-2
13	Combine	53.76	2	746	359,313	10, 11, 12			POST RUNOFF TO SIDE 1
14	SCS Runoff	26.02	2	750	169,141			P#7FP	PR-DA-2A-1
15	SCS Runoff	11.72	2	738	57,850				PR-DA-2A-2
16	SCS Runoff	6.766	2	736	32,097				PR-DA-2B
17	SCS Runoff	31.51	2	740	164,838		*******		PR-DA-2C
18	SCS Runoff	4.798	2	730	19,566				PR-DA-2D
19	SCS Runoff	6.794	2	732	30,101			******	PR-DA-2E
20	Combine	80.99	2	740	473,592	14, 15, 16,	17, 1 <del>8, 19</del>		POST RUNOFF TO SIDE 2
21	Reservoir	33.70	2	770	239,854	11	439.38	50,728	DETENTION BASIN 1
22	Combine	36.92	2	764	307,115	10, 12, 21			POST TO SIDE 1 W DETENTION
23	Combine	115.40	2	746	798,175	3, 9,			exist total site runoff
24	Reservoir	16.88	2	774	147,275	14	423.66	37,651	DETENTION BASIN 2
25	Combine	66,82	2	738	451,726	15, 16, 17,	18, 19 <del>, 2</del> 4		POST TO SIDE 2 W DETENTION
26	Combine	97.98	2	740	758,840	22, 25	*******		POST TOTAL SITE RUNOFF
flar	ders storm w	vater.gpw			Return F	Period: 25 \	/ear	Tuesday,	Aug 22, 2023

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to peak (min)	Hyd. volume (cuft)	inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph description
1	SCS Runoff	18.86	2	726	64,880	44 ha ha ha		450775	ex-da-1a
2	SCS Runoff	47.50	2	756	335,980			******	ex-da-1b
3	Combine	51.25	2	752	400,860	1, 2			exist runoff to side 1
4	SCS Runoff	39.76	2	750	258,024				ex-da-2a
5	SCS Runoff	8.028	2	736	38,233		#BN===		ex-da-2b
6	SCS Runoff	38.38	2	740	201,470		******		ex-da-2c
7	SCS Runoff	5.696	2	730	23,306				ex-da-2d
8	SCS Runoff	8.070	2	732	35,855			70000	ex-da-2e
9	Combine	90.88	2	742	556,886	4, 5, 6, 7, 8			exist runoff to side 2
10	SCS Runoff	4.227	2	732	18,781				PR-DA-1A
11	SCS Runoff	54.06	2	750	350,849				PR-DA-1B-1
12	SCS Runoff	13.74	2	732	61,169		845535		PR-DA-1B-2
13	Combine	64.42	2	746	430,799	10, 11, 12			POST RUNOFF TO SIDE 1
14	SCS Runoff	30.82	2	750	200,921				PR-DA-2A-1
15	SCS Runoff	13.91	2	738	68,908			******	PR-DA-2A-2
16	SCS Runoff	8.028	2	736	38,233				PR-DA-2B
17	SCS Runoff	37.40	2	740	196,348				PR-DA-2C
18	SCS Runoff	5.696	2	730	23,306	70	i		PR-DA-2D
19	SCS Runoff	8.070	2	732	35,855	4974 PFSH			PR-DA-2E
20	Combine	96.11	2	740	563,570	14, 15, 16,	17, 18 <del>, 1</del> 9		POST RUNOFF TO SIDE 2
21	Reservoir	39,18	2	772	294,192	11	439.91	62,011	DETENTION BASIN 1
22	Combine	42.79	2	764	374,142	10, 12, 21			POST TO SIDE 1 W DETENTION
23	Combine	138.25	2	746	957,747	3, 9,			exist total site runoff
24	Reservoir	20.97	2	772	176,890	14	424.07	43,391	DETENTION BASIN 2
25	Combine	79.13	2	738	539,538	15, 16, 17,	18, 19 <del>,-24</del>	*******	POST TO SIDE 2 W DETENTION
26	Combine	117.87	2	742	913,680	22, 25			POST TOTAL SITE RUNOFF
 flar	nders storm w	vater.gpw			Return F	Period: 50 \	/ear	Tuesday,	Aug 22, 2023

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to peak (mln)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph description		
1	SCS Runoff	22,12	2	726	76,490				ex-da-1a		
2	SCS Runoff	56.67	2	756	401,133	2000		***************************************	ex-da-1b		
3	Combine	61.09	2	752	477,623	1, 2		#50000	exist runoff to side 1		
4	SCS Runoff	47.14	2	750	306,467				ex-da-2a		
5	SCS Runoff	9,418	2	736	45,075				ex-da-2b		
6	SCS Runoff	45.05	2	740	237,522		***************************************		ex-da-2c		
7	SCS Runoff	6.685	2	730	27,476				ex-da-2d		
8	SCS Runoff	9.476	2	732	42,271				ex-da-2e		
9	Combine	107.13	2	742	658,810	4, 5, 6, 7, 8			exist runoff to side 2		
10	SCS Runoff	4.964	2	732	22,142		 		PR-DA-1A		
11	SCS Runoff	64.10	2	750	416,719				PR-DA-1B-1		
12	SCS Runoff	16.09	2	732	71,942				PR-DA-1B-2		
13	Combine	76.22	2	746	510,804	10, 11, 12			POST RUNOFF TO SIDE 1		
14	SCS Runoff	36.10	2	750	236,308				PR-DA-2A-1		
15	SCS Runoff	16.31	2	738	81,239				PR-DA-2A-2		
16	SCS Runoff	9.418	2	736	45,075				PR-DA-2B		
17	SCS Runoff	43.90	2	740	231,483				PR-DA-2C		
18	SCS Runoff	6.685	2	730	27,476				PR-DA-2D		
19	SCS Runoff	9,476	2	732	42,271	HNNA			PR-DA-2E		
20	Combine	112.76	2	740	663,851	14, 15, 16,	17, 18 <del>, 19</del>	71.75 P. 14.75	POST RUNOFF TO SIDE 2		
21	Reservoir	44.50	2	772	355,632	11	440.50	75,997	DETENTION BASIN 1		
22	Combine	48.52	2	764	449,716	10, 12, 21	200212		POST TO SIDE 1 W DETENTION		
23	Combine	163.56	2	746	1,136,432	3, 9,			exist total site runoff		
24	Reservoir	25.37	2	772	210,043	14	424.46	49,503	DETENTION BASIN 2		
25	Combine	93.18	2	738	637,586	15, 16, 17,	18, 19 <del>, 24</del>	MODERAL	POST TO SIDE 2 W DETENTION		
26	Combine	139.39	2	740	1,087,302	22, 25			POST TOTAL SITE RUNOFF		
flanders storm water.gpw				Return P	eriod: 100	Year	Tuesday,	Tuesday, Aug 22, 2023			

Hydraflow Hydrographs by Intelisolve v9.02

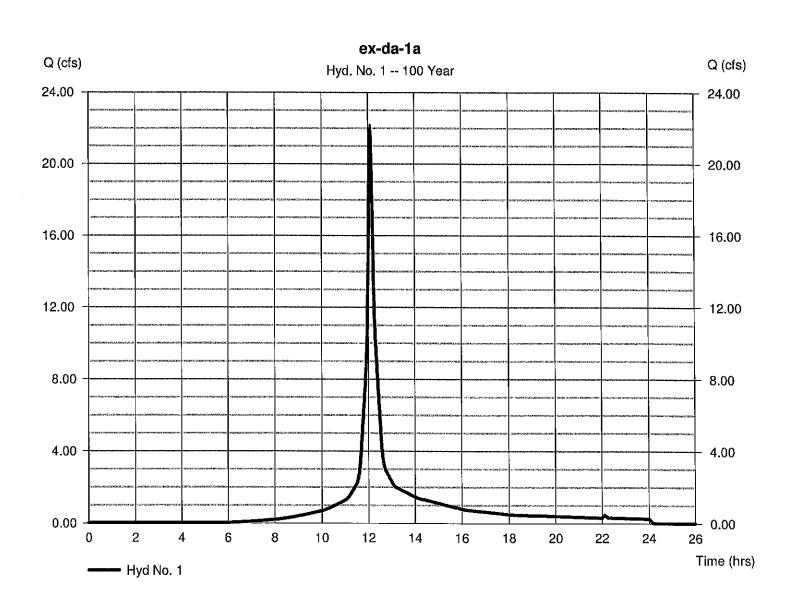
Tuesday, Aug 22, 2023

#### Hyd. No. 1

ex-da-1a

Hydrograph type = SCS Runoff Peak discharge = 22.12 cfsStorm frequency = 100 yrsTime to peak = 12.10 hrs Time interval = 2 min Hyd. volume = 76,490 cuftDrainage area = 3.800 acCurve number = 79\*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 9.00 min= TR55 Total precip. = 8.04 inDistribution = Type III Storm duration = 24 hrs Shape factor = 484

<sup>\*</sup> Composite (Area/CN) =  $[(2.100 \times 77) + (0.400 \times 83) + (0.700 \times 79) + (0.600 \times 84)] / 3.800$ 



Hydraflow Hydrographs by Intelisolve v9.02

Hyd. No. 1

ex-da-1a

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>	
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 150.0 = 3.34 = 55.00		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00			
Travel Time (min)	= 7.72	+	0.00	+	0.00	=	7.72	
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 445.00 = 13.00 = Unpaved = 5.82	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00			
Travel Time (min)	= 1.27	+	0.00	+	0.00	=	1.27	
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s) Flow length (ft)	= 0.00 = 0.00 = 0.00 = 0.015 = 0.00 = 0.0		0.00 0.00 0.00 0.015 0.00 0.0		0.00 0.00 0.00 0.015 0.00 0.0			
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00	
Total Travel Time, Tc								

Hydraflow Hydrographs by Intelisolve v9.02

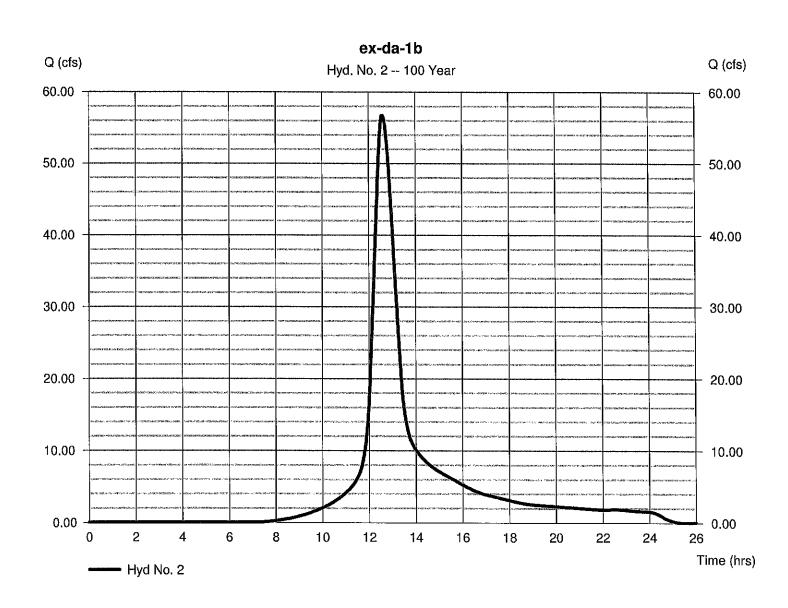
Tuesday, Aug 22, 2023

#### Hyd. No. 2

ex-da-1b

Hydrograph type = SCS Runoff Peak discharge = 56.67 cfsStorm frequency = 100 yrsTime to peak = 12.60 hrsTime interval = 2 minHyd. volume = 401,133 cuftDrainage area = 22.100 acCurve number = 74\*Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) = 51.80 min Tc method = TR55 Total precip. Distribution = 8.04 in= Type III Storm duration = 24 hrs Shape factor = 484

<sup>\*</sup> Composite (Area/CN) =  $[(2.100 \times 66) + (0.200 \times 67) + (1.400 \times 60) + (9.800 \times 73) + (8.600 \times 79)] / 22.100$ 



Hydraflow Hydrographs by Intelisolve v9.02

Hyd. No. 2

ex-da-1b

<u>Description</u>		<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>	
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= =	0.400 300.0 3.34 2.30		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00			
Travel Time (min)	=	47.87	+	0.00	+	0.00	=	47.87	
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	=	1205.00 9.80 Unpaved 5.05		0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00			
Travel Time (min)	=	3.98	+	0.00	+	0.00	=	3.98	
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s) Flow length (ft)	= =	0.00 0.00 0.00 0.015 0.00 0.0		0.00 0.00 0.00 0.015 0.00 0.0		0.00 0.00 0.00 0.015 0.00			
Travel Time (min)	=	0.00	+	0.00	+	0.00	=	0.00	
Total Travel Time, Tc									

Hydraflow Hydrographs by Intelisolve v9.02

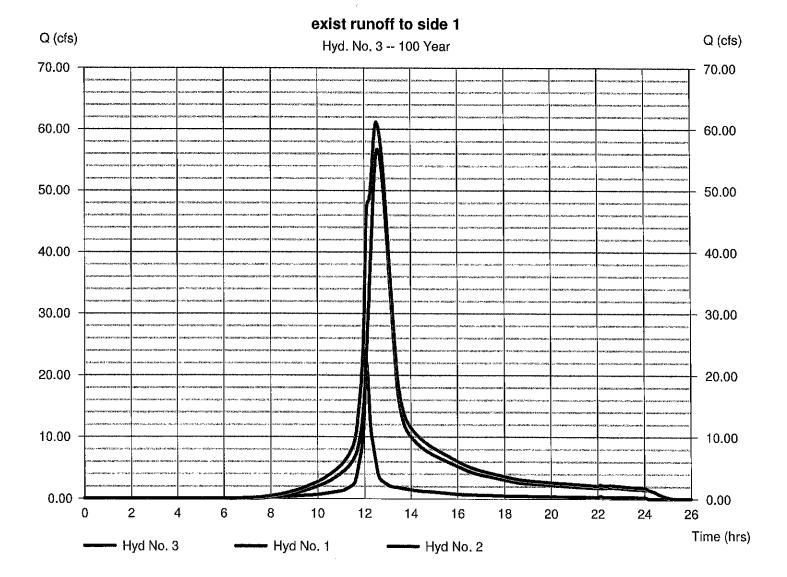
Tuesday, Aug 22, 2023

#### Hyd. No. 3

exist runoff to side 1

Hydrograph type = Combine Storm frequency = 100 yrs Time interval = 2 min

Time interval = 2 min Inflow hyds. = 1, 2 Peak discharge = 61.09 cfs Time to peak = 12.53 hrs Hyd. volume = 477,623 cuft Contrib. drain. area = 25.900 ac



Hydraflow Hydrographs by Intelisolve v9.02

Tuesday, Aug 22, 2023

#### Hyd. No. 4

ex-da-2a

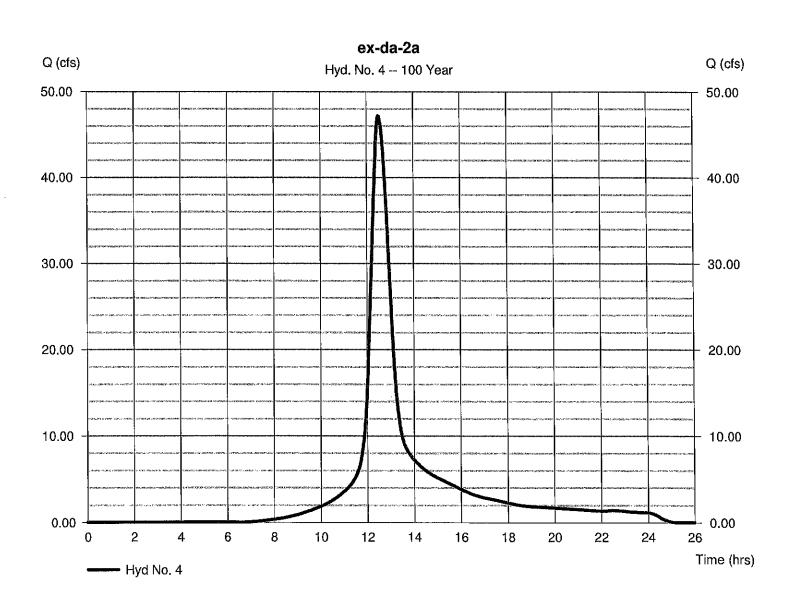
Hydrograph type = SCS Runoff Storm frequency = 100 yrsTime interval = 2 min Drainage area = 16.400 acBasin Slope = 0.0 %Tc method = TR55 Total precip. = 8.04 inStorm duration = 24 hrs

Peak discharge = 47.14 cfs
Time to peak = 12.50 hrs
Hyd. volume = 306,467 cuft

Curve number = 76\* Hydraulic length = 0 ft

Time of conc. (Tc) = 46.90 min
Distribution = Type III
Shape factor = 484

<sup>\*</sup> Composite (Area/CN) =  $[(1.200 \times 73) + (1.700 \times 60) + (11.900 \times 79) + (1.200 \times 66) + (0.400 \times 77)] / 16.400$ 



Hydraflow Hydrographs by Intelisolve v9.02

Hyd. No. 4

ex-da-2a

<b>Description</b>		<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>	
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	=	0.400 300.0 3.34 3.30		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00			
Travel Time (min)	=	41.43	+	0.00	+	0.00	=	41.43	
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	=	1570.00 8.80 Unpaved 4.79		0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00			
Travel Time (min)	=	5.47	+	0.00	+	0.00	=	5.47	
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s) Flow length (ft)	= =	0.00 0.00 0.00 0.015 0.00 0.0		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015 0.00 0.0			
Travel Time (min)	=	0.00	+	0.00	+	0.00	=	0.00	
Total Travel Time, Tc									

Hydraflow Hydrographs by Intelisoive v9.02

Tuesday, Aug 22, 2023

#### Hyd. No. 5

ex-da-2b

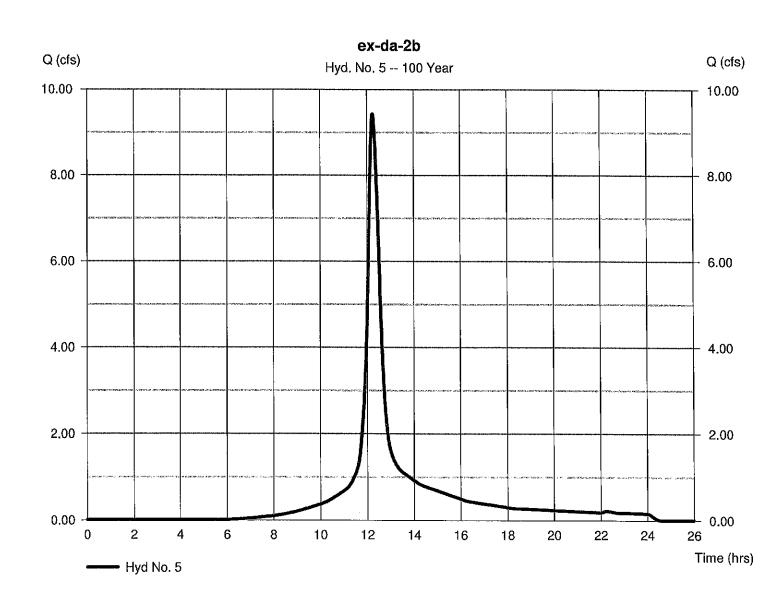
Hydrograph type = SCS Runoff Storm frequency = 100 yrsTime interval  $= 2 \min$ Drainage area = 2.200 acBasin Slope = 0.0 %Tc method = TR55Total precip. = 8.04 inStorm duration = 24 hrs

Peak discharge = 9.418 cfs
Time to peak = 12.27 hrs
Hyd. volume = 45,075 cuft

Curve number  $= 79^*$ Hydraulic length = 0 ft

Time of conc. (Tc) = 21.70 min
Distribution = Type III
Shape factor = 484

<sup>\*</sup> Composite (Area/CN) = [(2.200 x 79)] / 2.200



Hydraflow Hydrographs by Intelisolve v9.02

Hyd. No. 5

ex-da-2b

<u>Description</u>		A		<u>B</u>		<u>C</u>		<u>Totals</u>	
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	=	0.400 150.0 3.34 5.30		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00			
Travel Time (min)		19.69	+	0.00	+	0.00	=	19.69	
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	=	278.00 2.10 Unpaved 2.34		0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00			
Travel Time (min)	=	1.98	+	0.00	+	0.00	=	1.98	
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s) Flow length (ft)	= = =	0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015 0.00 0.0		0.00 0.00 0.00 0.015 0.00			
Travel Time (min)	=	0.00	+	0.00	+	0.00	=	0.00	
Total Travel Time, Tc									

Hydraflow Hydrographs by Intelisolve v9.02

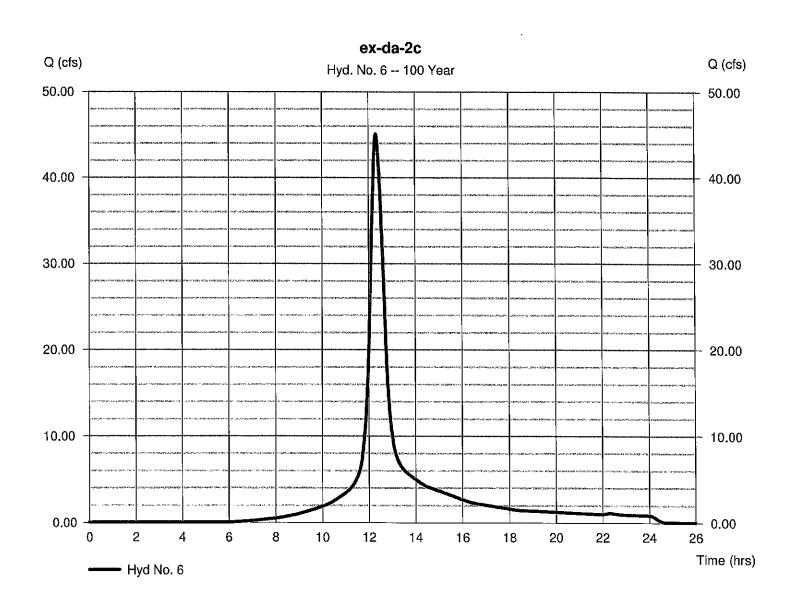
Tuesday, Aug 22, 2023

#### Hyd. No. 6

ex-da-2c

Hydrograph type = SCS Runoff Peak discharge = 45.05 cfsStorm frequency Time to peak = 100 yrs $= 12.33 \, hrs$ Time interval Hyd. volume = 2 min= 237,522 cuft Drainage area = 11.800 acCurve number = 79\*Basin Slope = 0.0 % Hydraulic length = 0 ftTime of conc. (Tc) = 29.30 minTc method = TR55 Total precip. = 8.04 inDistribution = Type III Storm duration = 24 hrs Shape factor = 484

<sup>\*</sup> Composite (Area/CN) = [(0.500 x 73) + (11.300 x 79)] / 11.800



Hydraflow Hydrographs by Intelisolve v9.02

Hyd. No. 6

ex-da-2c

Description	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>	
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 300.0 = 3.34 = 11.30		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00			
Travel Time (min)	= 25.32	+	0.00	+	0.00	=	25.32	
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 1110.00 = 8.50 = Unpaved = 4.70	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00			
Travel Time (min)	= 3.93	+	0.00	+	0.00	=	3.93	
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s) Flow length (ft)	= 0.00 = 0.00 = 0.00 = 0.015 = 0.00 = 0.0		0.00 0.00 0.00 0.015 0.00 0.0		0.00 0.00 0.00 0.015 0.00			
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00	
Total Travel Time, Tc								

Hydraflow Hydrographs by Intelisolve v9.02

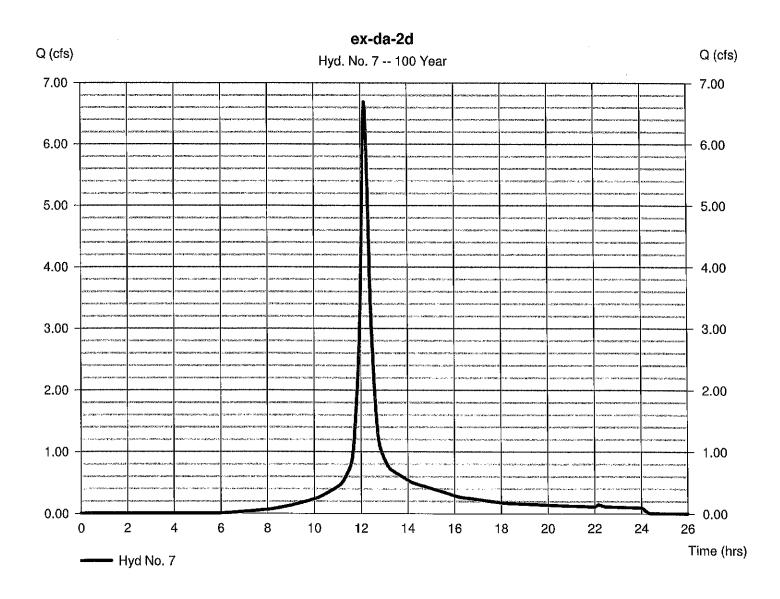
Tuesday, Aug 22, 2023

#### Hyd. No. 7

ex-da-2d

Hydrograph type = SCS Runoff Peak discharge = 6.685 cfsStorm frequency Time to peak = 100 yrs $= 12.17 \, hrs$ Time interval Hyd. volume = 2 min= 27,476 cuftDrainage area = 1.400 acCurve number = 79\*Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) = 15.80 minTc method = TR55Total precip. Distribution = 8.04 inType III Storm duration = 24 hrs Shape factor = 484

<sup>\*</sup> Composite (Area/CN) = [(1.400 x 79)] / 1.400



Hydraflow Hydrographs by Intelisolve v9.02

Hyd. No. 7

ex-da-2d

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>	
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 150.0 = 3.34 = 10.60		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00			
Travel Time (min)	= 14.92	+	0.00	+	0.00	=	14.92	
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 325.00 = 13.80 = Unpaved = 5.99		0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00			
Travel Time (min)	= 0.90	+	0.00	+	0.00	=	0.90	
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s) Flow length (ft)	= 0.00 = 0.00 = 0.00 = 0.015 = 0.00 = 0.0		0.00 0.00 0.00 0.015 0.00 0.0		0.00 0.00 0.00 0.015 0.00			
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00	
Total Travel Time, Tc								

Hydraflow Hydrographs by Intelisolve v9.02

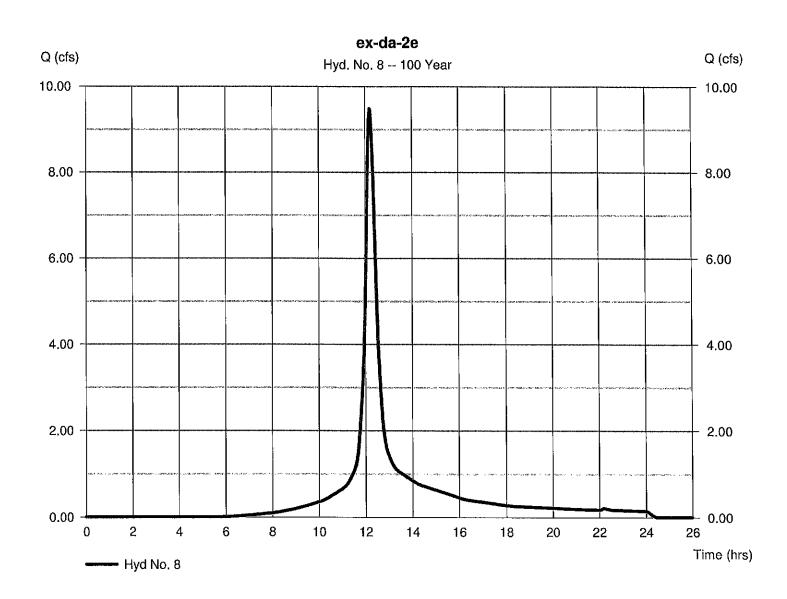
Tuesday, Aug 22, 2023

#### Hyd. No. 8

ex-da-2e

Hydrograph type = SCS Runoff Peak discharge = 9.476 cfsStorm frequency Time to peak = 100 yrs= 12.20 hrsTime interval = 2 min Hyd. volume = 42,271 cuftDrainage area = 2.100 acCurve number = 79\* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55  $= 18.00 \, \text{min}$ Total precip. = 8.04 inDistribution = Type III Storm duration = 24 hrs Shape factor = 484

<sup>\*</sup> Composite (Area/CN) =  $[(2.100 \times 79)] / 2.100$ 



Hyd. No. 8

ex-da-2e

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>	
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 150.0 = 3.34 = 8.00		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00			
Travel Time (min)	= 16.70	+	0.00	+	0.00	=	16.70	
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 500.00 = 16.00 = Unpave = 6.45		0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00			
Travel Time (min)	= 1.29	+	0.00	+	0.00	=	1.29	
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s) Flow length (ft)	= 0.00 = 0.00 = 0.00 = 0.015 = 0.00 = 0.0		0.00 0.00 0.00 0.015 0.00 0.0		0.00 0.00 0.00 0.015 0.00			
Travel Time (min)	<b>= 0.00</b>	+	0.00	+	0.00	=	0.00	
Total Travel Time, Tc								

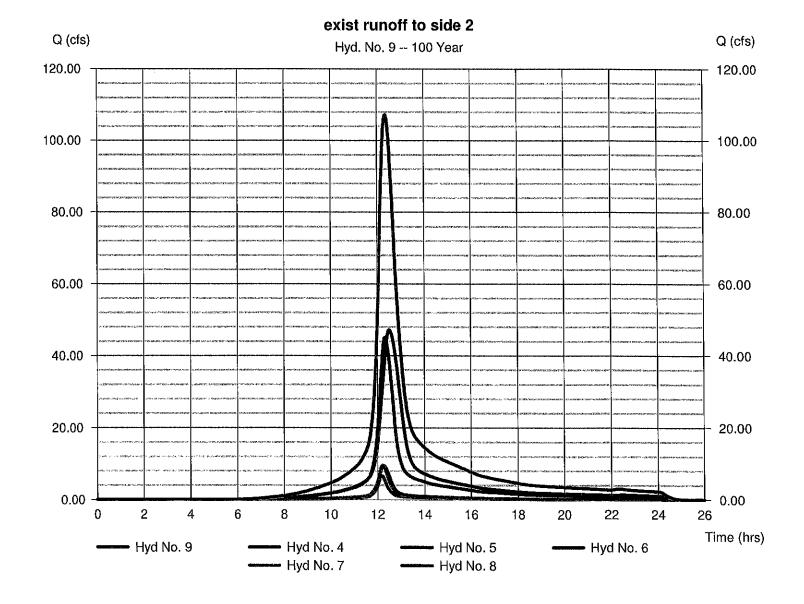
Hydraflow Hydrographs by Intelisolve v9.02

Tuesday, Aug 22, 2023

### Hyd. No. 9

exist runoff to side 2

Hydrograph type = Combine Storm frequency = 100 yrs Time interval = 2 min Inflow hyds. = 4, 5, 6, 7, 8 Peak discharge = 107.13 cfs Time to peak = 12.37 hrs Hyd. volume = 658,810 cuft Contrib. drain. area = 33.900 ac



Hydraflow Hydrographs by Intelisoive v9.02

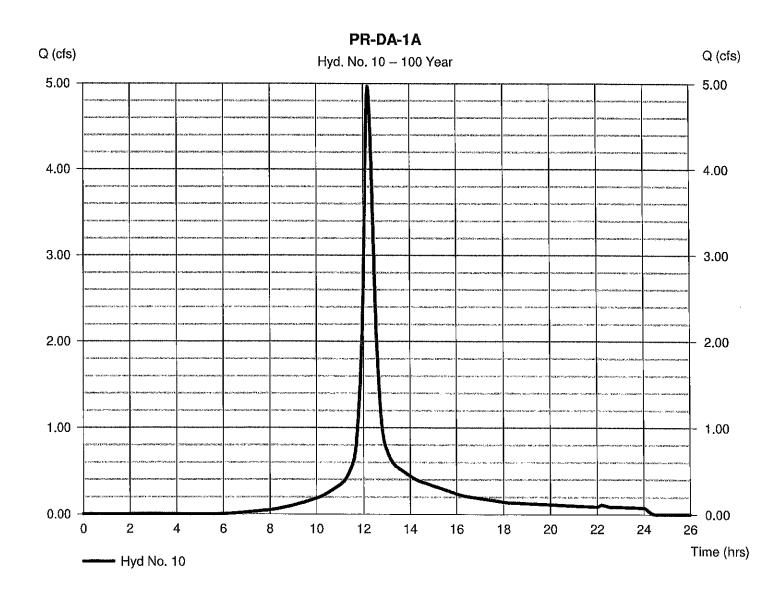
Tuesday, Aug 22, 2023

### Hyd. No. 10

PR-DA-1A

= SCS Runoff Hydrograph type Peak discharge = 4.964 cfsStorm frequency = 100 yrsTime to peak  $= 12.20 \, hrs$ Time interval = 2 min Hyd. volume = 22,142 cuftDrainage area = 1.100 acCurve number = 79\*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 17.70 min= TR55 Total precip. Distribution = 8.04 in= Type III Storm duration = 24 hrs Shape factor = 484

<sup>\*</sup> Composite (Area/CN) =  $[(0.200 \times 98) + (0.260 \times 79) + (0.640 \times 73)] / 1.100$ 



### Hyd. No. 10

PR-DA-1A

<u>Description</u>	A		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 150.0 = 3.34 = 8.00		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 16.70	+	0.00	+	0.00	=	16.70
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 350.00 = 16.00 = Unpay = 6.45		22.00 2.00 Paved 2.87		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.90	+	0.13	+	0.00	=	1.03
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s) Flow length (ft)	= 0.00 = 0.00 = 0.00 = 0.015 = 0.00 = 0.0		0.00 0.00 0.00 0.015 0.00 0.0		0.00 0.00 0.00 0.015 0.00 0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							

Hydraflow Hydrographs by Intelisolve v9.02

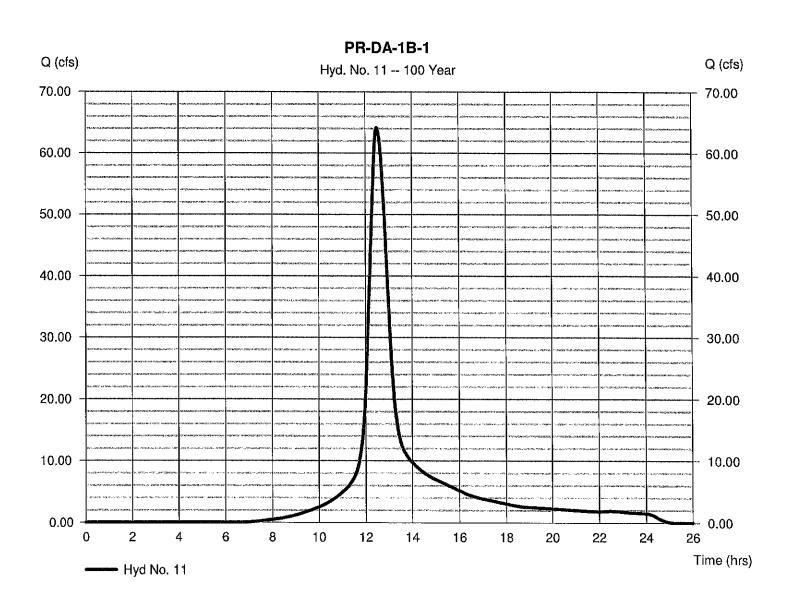
Tuesday, Aug 22, 2023

### Hyd. No. 11

PR-DA-1B-1

Hydrograph type = SCS Runoff Peak discharge = 64.10 cfsStorm frequency = 100 yrsTime to peak = 12.50 hrsTime interval = 2 min Hyd. volume = 416,719 cuftDrainage area = 22.300 acCurve number = 76\* Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55  $= 44.50 \, \text{min}$ Total precip. = 8.04 inDistribution = Type III Storm duration = 24 hrs Shape factor = 484

<sup>\*</sup> Composite (Area/CN) =  $[(3.300 \times 98) + (2.630 \times 61) + (4.000 \times 80) + (0.770 \times 79) + (9.500 \times 73) + (2.100 \times 66)] / 22.300$ 



## Hyd. No. 11

PR-DA-1B-1

<u>Description</u>		<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	=	0.400 300.0 3.34 3.60		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	=	40.01	+	0.00	+	0.00	=	40.01
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	=	1253.00 8.90 Unpaved 4.81		22.00 2.00 Paved 2.87		0.00 0.00 Paved 0.00		
Travel Time (min)	=	4.34	+	0.13	+	0.00	=	4.47
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s) Flow length (ft)		0.00 0.00 0.00 0.015 0.00 0.0		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015 0.00 0.0		
Travel Time (min)	=	0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc								

Hydraflow Hydrographs by Intelisolve v9.02

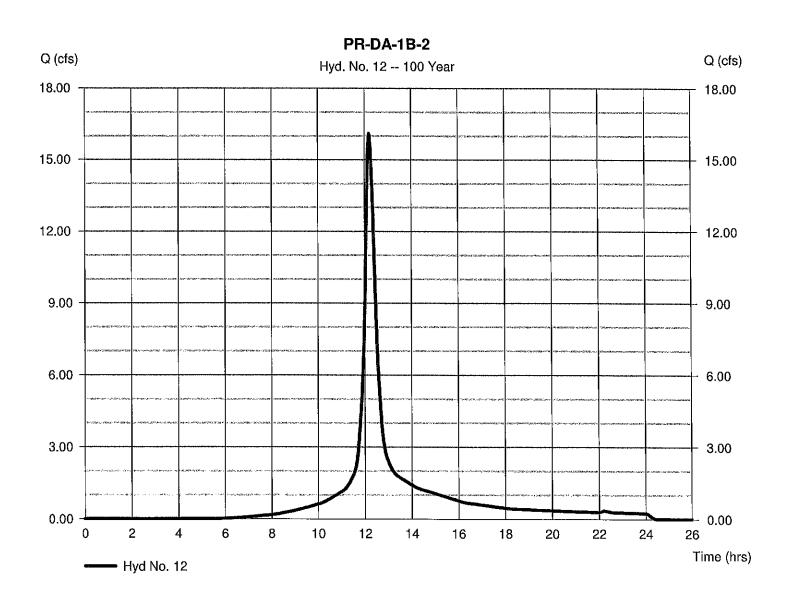
Tuesday, Aug 22, 2023

### Hyd. No. 12

PR-DA-1B-2

Hydrograph type = SCS Runoff Peak discharge = 16.09 cfsStorm frequency = 100 yrsTime to peak = 12.20 hrs Time interval = 2 min Hyd. volume = 71,942 cuftDrainage area = 3.500 acCurve number = 80\*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55Time of conc. (Tc) = 16.80 minTotal precip. = 8.04 inDistribution Type III Storm duration = 24 hrs Shape factor = 484

<sup>\*</sup> Composite (Area/CN) =  $[(0.080 \times 98) + (0.510 \times 80) + (2.910 \times 79)] / 3.500$ 



Hyd. No. 12

PR-DA-1B-2

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>	
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 150.0 = 3.34 = 11.30		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00			
Travel Time (min)	= 14.54	+	0.00	+	0.00	=	14.54	
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)  Travel Time (min)	= 555.00 = 6.60 = Unpaved = 4.15 = <b>2.23</b>	<b>.</b>	0.00 0.00 Paved 0.00	+	0.00 0.00 Paved 0.00	=	2,23	
maver rime (min)	- 4.67	т	0.00	T	0.00	=	<i>L.</i>	
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s) Flow length (ft)	= 0.00 = 0.00 = 0.00 = 0.015 = 0.00 = 0.0		0.00 0.00 0.00 0.015 0.00 0.0		0.00 0.00 0.00 0.015 0.00 0.0			
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00	
Total Travel Time, Tc								

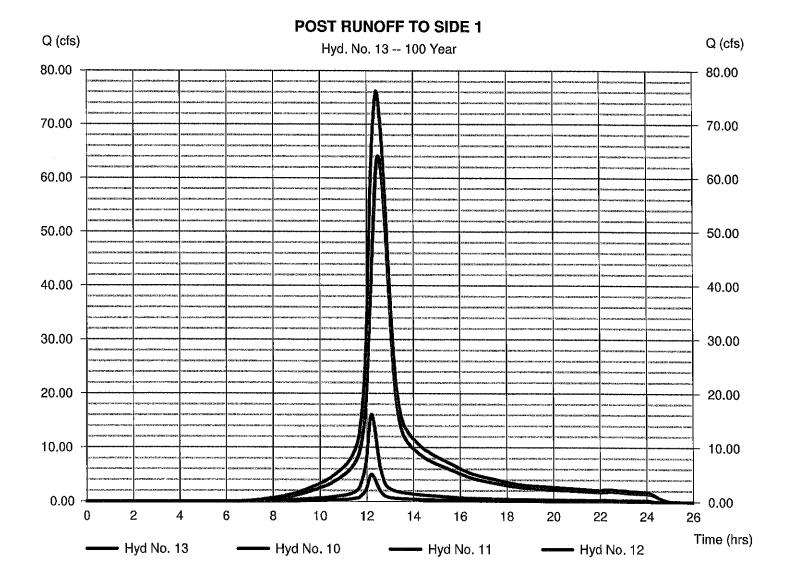
Hydraflow Hydrographs by Intelisolve v9.02

Tuesday, Aug 22, 2023

### Hyd. No. 13

### POST RUNOFF TO SIDE 1

Hydrograph type = Combine Storm frequency = 100 yrs Time interval = 2 min Inflow hyds. = 10, 11, 12 Peak discharge = 76.22 cfs Time to peak = 12.43 hrs Hyd. volume = 510,804 cuft Contrib. drain. area = 26.900 ac



Hydraflow Hydrographs by Intelisolve v9.02

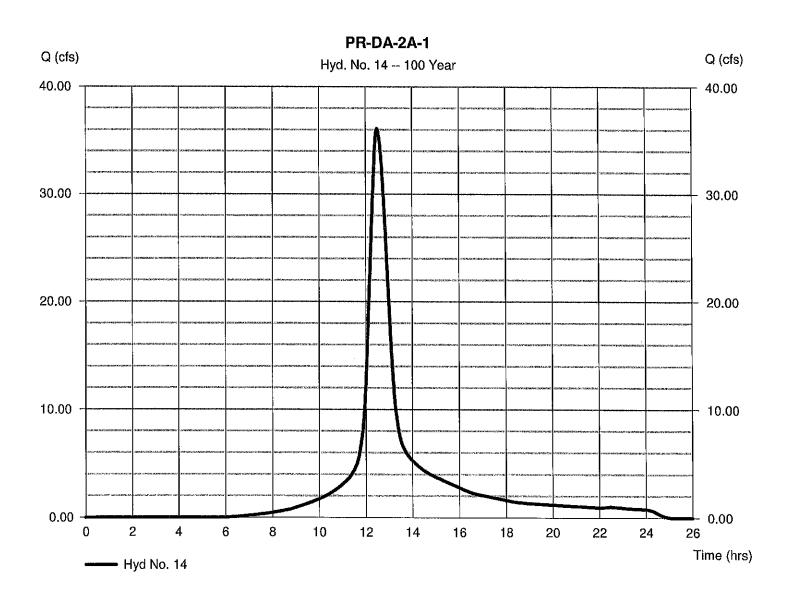
Tuesday, Aug 22, 2023

### Hyd. No. 14

PR-DA-2A-1

Hydrograph type = SCS Runoff Peak discharge = 36.10 cfsStorm frequency = 100 yrsTime to peak = 12.50 hrsTime interval = 2 min Hyd. volume = 236,308 cuft Drainage area = 11.600 acCurve number = 80\*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 47.10 min= TR55Total precip. = 8.04 inDistribution = Type III Storm duration = 24 hrs Shape factor = 484

<sup>\*</sup> Composite (Area/CN) =  $[(2.000 \times 98) + (0.310 \times 60) + (6.600 \times 79) + (1.090 \times 73) + (0.400 \times 77) + (1.200 \times 66)] / 11.600$ 



### Hyd. No. 14

PR-DA-2A-1

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>	
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 300.0 = 3.34 = 3.30		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00			
Travel Time (min)	= 41.43	3 +	0.00	+	0.00	=	41.43	
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 1578 = 8.60 = Unpa = 4.73		22.00 2.00 Paved 2.87		0.00 0.00 Paved 0.00			
Travel Time (min)	= 5.56	+	0.13	+	0.00	=	5.69	
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s) Flow length (ft)	= 0.00 = 0.00 = 0.00 = 0.018 = 0.00 = 0.0	5	0.00 0.00 0.00 0.015 0.00 0.0		0.00 0.00 0.00 0.015 0.00 0.0			
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00	
Total Travel Time, Tc								

Hydraflow Hydrographs by Intelisoive v9.02

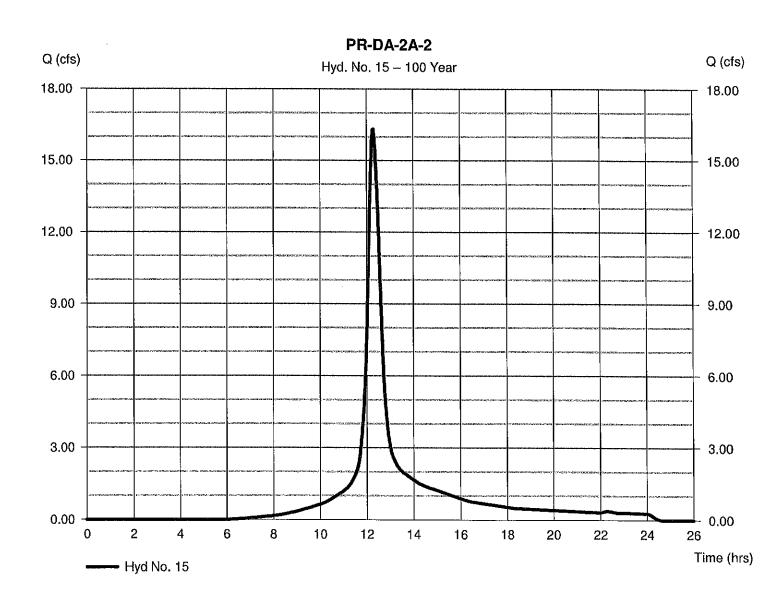
Tuesday, Aug 22, 2023

### Hyd. No. 15

PR-DA-2A-2

Hydrograph type = SCS Runoff Peak discharge = 16.31 cfsStorm frequency = 100 yrsTime to peak  $= 12.30 \, hrs$ Time interval = 2 min Hyd. volume = 81,239 cuftDrainage area = 4.100 acCurve number = 79\*Basin Slope = 0.0 % Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 24.00 \, \text{min}$ Total precip. = 8.04 inDistribution = Type III Storm duration = 24 hrs Shape factor = 484

<sup>\*</sup> Composite (Area/CN) =  $[(0.190 \times 74) + (0.060 \times 80) + (0.220 \times 73) + (3.440 \times 79) + (0.090 \times 98) + (0.100 \times 89)] / 4.100$ 



Hyd. No. 15

PR-DA-2A-2

<u>Description</u>	A		<u>B</u>		<u>C</u>		<u>Totals</u>	
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 300.0 = 3.34 = 16.00		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00			
Travel Time (min)	= 22.03	+	0.00	+	0.00	=	22.03	
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 430.00 = 5.10 = Unpave = 3.64	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00			
Travel Time (min)	= 1.97	+	0.00	+	0.00	=	1.97	
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s) Flow length (ft)	= 0.00 = 0.00 = 0.00 = 0.015 = 0.00 = 0.0		0.00 0.00 0.00 0.015 0.00 0.0		0.00 0.00 0.00 0.015 0.00 0.0			
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00	
Total Travel Time, Tc								

Hydraflow Hydrographs by Intelisolve v9.02

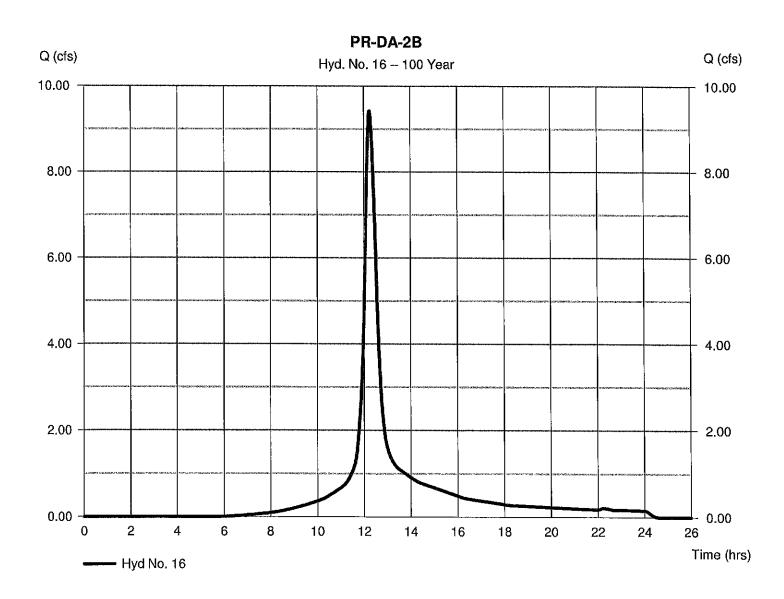
Tuesday, Aug 22, 2023

### Hyd. No. 16

PR-DA-2B

Hydrograph type = SCS Runoff Peak discharge = 9.418 cfsStorm frequency = 100 yrsTime to peak = 12.27 hrsTime interval = 2 minHyd. volume = 45,075 cuftDrainage area = 2.200 acCurve number = 79\*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55Time of conc. (Tc)  $= 21.80 \, \text{min}$ Total precip. = 8.04 inDistribution = Type III Storm duration = 24 hrs Shape factor = 484

<sup>\*</sup> Composite (Area/CN) = [(2.000 x 79) + (0.200 x 84)] / 2.200



Hyd. No. 16

PR-DA-2B

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>	
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 150.0 = 3.34 = 6.60		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00			
Travel Time (min)	= 18.03	+	0.00	+	0.00	=	18.03	
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 640.00 = 3.10 = Unpaved = 2.84	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00			
Travel Time (min)	= 3.75	+	0.00	+	0.00	=	3.75	
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s) Flow length (ft)	= 0.00 = 0.00 = 0.00 = 0.015 = 0.00 = 0.0		0.00 0.00 0.00 0.015 0.00 0.0		0.00 0.00 0.00 0.015 0.00 0.0			
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00	
Total Travel Time, Tc								

Hydraflow Hydrographs by Intelisolve v9.02

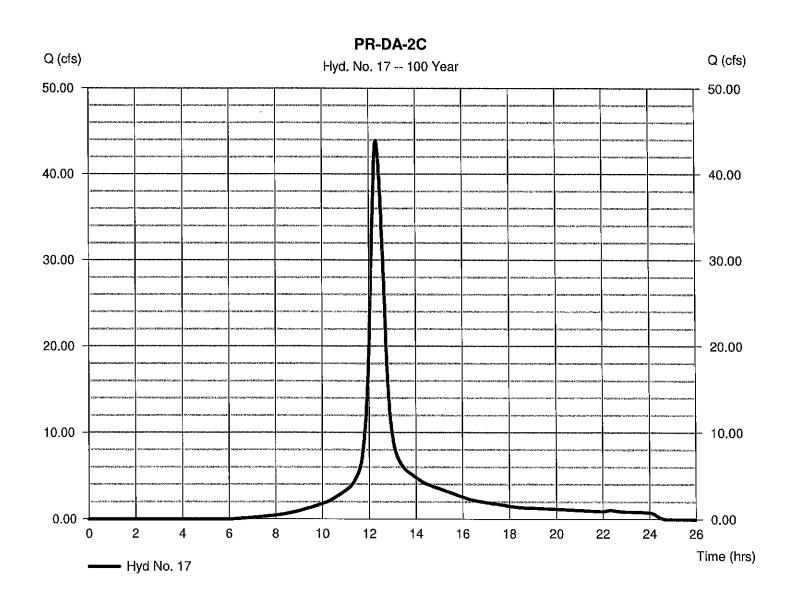
Tuesday, Aug 22, 2023

### Hyd. No. 17

PR-DA-2C

Hydrograph type = SCS Runoff Peak discharge = 43.90 cfsStorm frequency Time to peak = 100 yrs $= 12.33 \, hrs$ Time interval = 2 min Hyd. volume = 231,483 cuft Drainage area = 11.500 acCurve number = 79\* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55Time of conc. (Tc)  $= 29.30 \, \text{min}$ Total precip. = 8.04 inDistribution Type III Storm duration = 24 hrs Shape factor = 484

<sup>\*</sup> Composite (Area/CN) =  $[(0.170 \times 98) + (0.760 \times 80) + (1.070 \times 73) + (9.500 \times 79)] / 11.500$ 



Hyd. No. 17

PR-DA-2C

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>	
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 300.0 = 3.34 = 11.30		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00			
Travel Time (min)	= 25.32	+	0.00	+	0.00	=	25.32	
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 1110.00 = 8.50 = Unpave = 4.70	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00			
Travel Time (min)	= 3.93	+	0.00	+	0.00	=	3.93	
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s) Flow length (ft)	= 0.00 = 0.00 = 0.00 = 0.015 = 0.00 = 0.0		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015 0.00 0.0			
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00	
Total Travel Time, Tc								

Hydraflow Hydrographs by Intelisolve v9.02

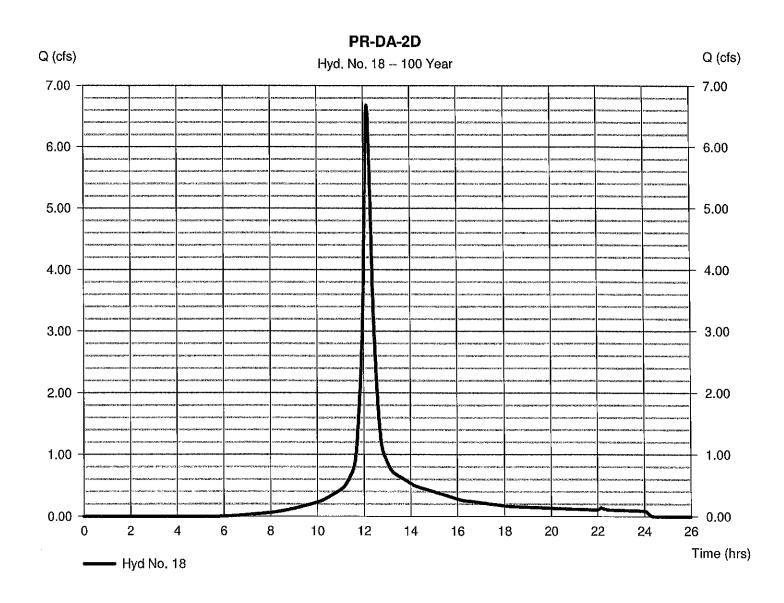
Tuesday, Aug 22, 2023

### Hyd. No. 18

PR-DA-2D

Hydrograph type = SCS Runoff Peak discharge = 6.685 cfsStorm frequency = 100 yrsTime to peak  $= 12.17 \, hrs$ Time interval = 2 minHyd. volume = 27,476 cuftDrainage area = 1.400 acCurve number = 79\*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 15.80 \, \text{min}$ Total precip. = 8.04 inDistribution = Type III Storm duration = 24 hrs Shape factor = 484

<sup>\*</sup> Composite (Area/CN) = [(1.400 x 79)] / 1,400



Hyd. No. 18

PR-DA-2D

<u>Description</u>		<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	=	0.400 150.0 3.34 10.60		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	=	14.92	+	0.00	+	0.00	=	14.92
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	=======================================	325.00 13.80 Unpaved 5.99		0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	=	0.90	+	0.00	+	0.00	=	0.90
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s) Flow length (ft)	=======================================	0.00 0.00 0.00 0.015 0.00 0.0		0.00 0.00 0.00 0.015 0.00 0.0		0.00 0.00 0.00 0.015 0.00 0.0		
Travel Time (min)	=	0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc								

Hydraflow Hydrographs by Intelisolve v9.02

Tuesday, Aug 22, 2023

### Hyd. No. 19

PR-DA-2E

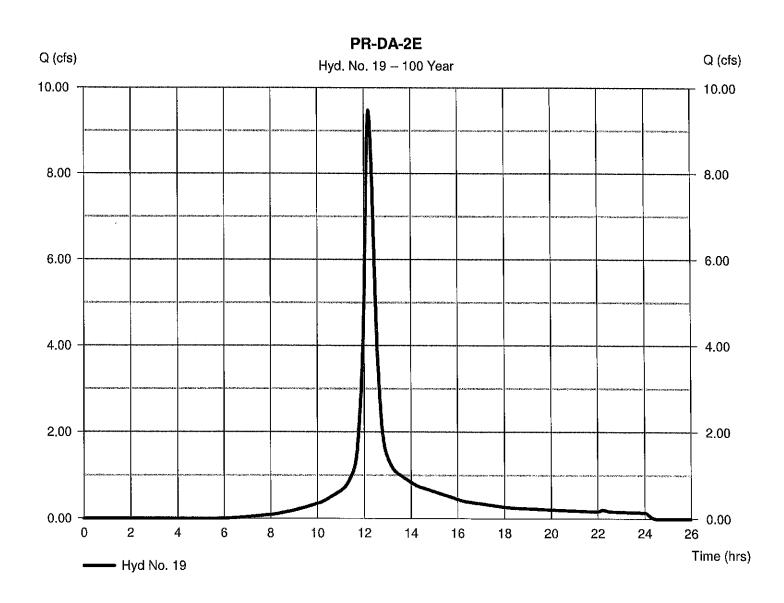
Hydrograph type = SCS Runoff Storm frequency = 100 yrsTime interval = 2 min Drainage area = 2.100 acBasin Slope = 0.0 %Tc method = TR55 Total precip. = 8.04 inStorm duration = 24 hrs

Peak discharge = 9.476 cfs
Time to peak = 12.20 hrs
Hyd. volume = 42,271 cuft

Curve number =  $79^*$ Hydraulic length = 0 ft Time of conc. (Tc) = 18.00 min

Distribution = Type III
Shape factor = 484

<sup>\*</sup> Composite (Area/CN) = [(2.100 x 79)] / 2.100



Hyd. No. 19

PR-DA-2E

<u>Description</u>		<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	=	0.400 150.0 3.34 8.00		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	=	16.70	+	0.00	+	0.00	=	16.70
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	=======================================	500.00 16.00 Unpaved 6.45		0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	=	1.29	+	0.00	+	0.00	=	1.29
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s) Flow length (ft)	= = =	0.00 0.00 0.00 0.015 0.00 0.0		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015 0.00		
Travel Time (min)	=	0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc								18.00 min

Hydraflow Hydrographs by Intelisolve v9.02

Tuesday, Aug 22, 2023

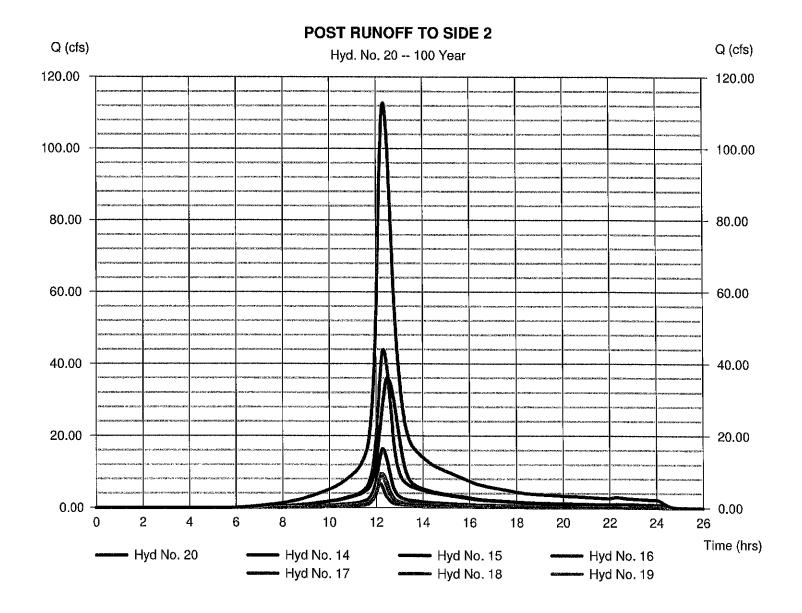
### Hyd. No. 20

### POST RUNOFF TO SIDE 2

Hydrograph type = Combine Storm frequency = 100 yrs Time interval = 2 min

Inflow hyds. = 14, 15, 16, 17, 18, 19

Peak discharge = 112.76 cfs
Time to peak = 12.33 hrs
Hyd. volume = 663,851 cuft
Contrib. drain. area = 32.900 ac



Hydraflow Hydrographs by Intelisolve v9.02

Tuesday, Aug 22, 2023

### Hyd. No. 21

### **DETENTION BASIN 1**

Hydrograph type = Reservoir Storm frequency = 100 yrs Time interval = 2 min

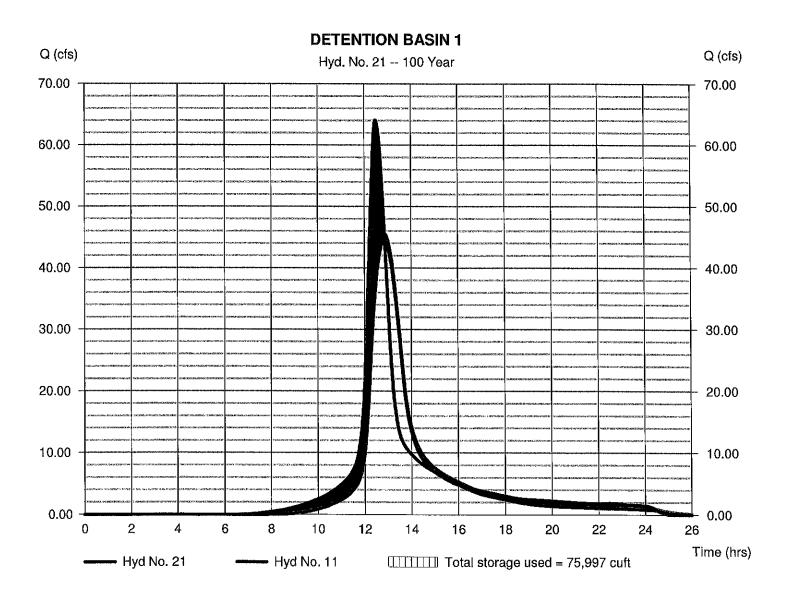
Inflow hyd. No. = 11 - PR-DA-1B-1

Reservoir name = POND 1

Peak discharge = 44.50 cfs Time to peak = 12.87 hrs Hyd. volume = 355,632 cuft

Max. Elevation = 440.50 ft Max. Storage = 75,997 cuft

Storage Indication method used. Exfiltration extracted from Outflow.



# **Pond Report**

Hydraflow Hydrographs by Intelisolve v9.02

Tuesday, Aug 22, 2023

Pond No. 1 - POND 1

#### **Pond Data**

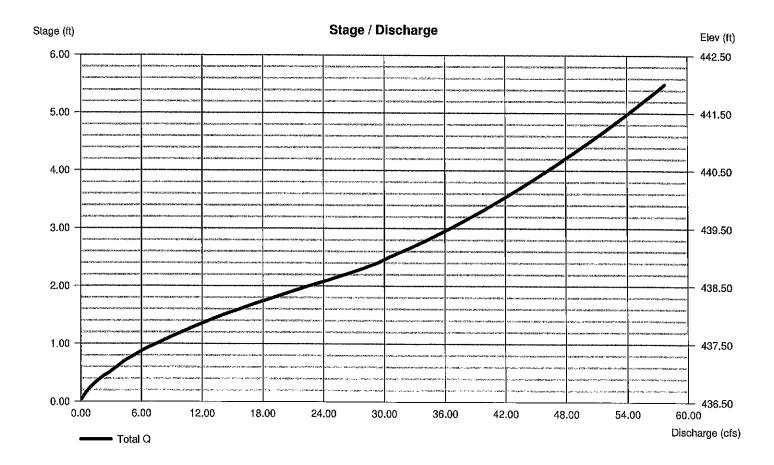
Contours - User-defined contour areas. Average end area method used for volume calculation. Begining Elevation = 436,50 ft

#### Stage / Storage Table

Stage (ft)	Stage (ft) Elevation (ft) Contour area (sqft)		Incr. Storage (cuft)	Total storage (cuft)		
0.00	436.50	14,061	0	0		
0.50	437.00	15,208	7,317	7.317		
1.50	438.00	17,630	16,419	23,736		
2.50	439.00	20,127	18,879	42,615		
3.50	440.00	22,720	21.424	64,038		
4.50	441.00	25,567	24,144	88,182		
5.50	442.00	28,507	27,037	115,219		

#### **Culvert / Orifice Structures Weir Structures** [A] [B] [C] [PrfRsr] [A] [B] [C] [D] Rise (in) = 30.0012.00 12.00 0.00 Crest Len (ft) Inactive 10.00 0.00 0.00 Span (in) = 30.0012.00 12.00 0.00 Crest El. (ft) = 440.00441.50 0.00 0.00 No. Barrels = 1 2 0 Welr Coeff. 1 = 3.332.60 3.33 3.33 Invert El. (ft) = 436.50436,50 438.00 0.00 Weir Type = Rect Broad Length (ft) = 60.000.00 0.00 0.00 Multi-Stage Yes = Yes No No Slope (%) = 2.000.00 0.00 n/a N-Value = .013.013 .013 n/a Orifice Coeff. = 0.600.60 0.60 0.60 Exfll.(in/hr) = 3.000 (by Contour) Multi-Stage = n/aYes Νo Nο TW Elev. (ft) = 0.00

Note: Culvert/Orifice outflows are analyzed under inlet and outlet control. Weir risers are checked for prifice conditions.



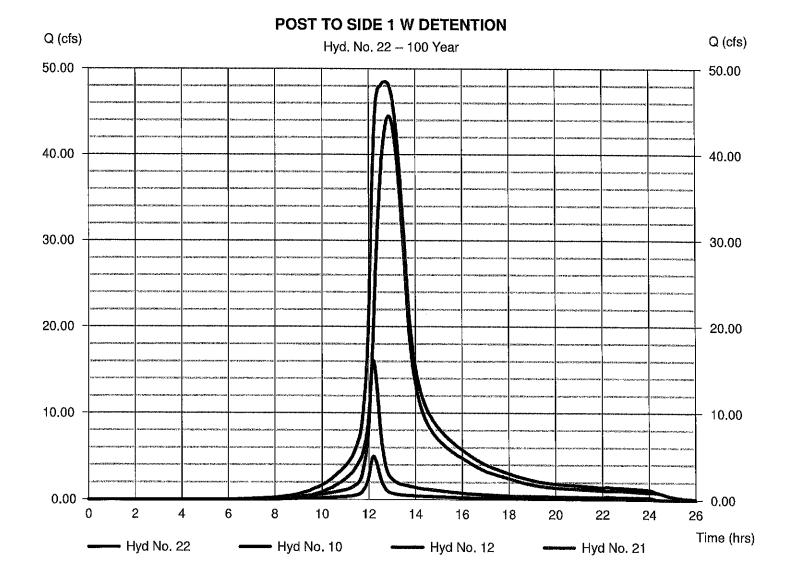
Hydraflow Hydrographs by Intelisolve v9.02

Tuesday, Aug 22, 2023

### Hyd. No. 22

### POST TO SIDE 1 W DETENTION

Hydrograph type = Combine Storm frequency = 100 yrs Time interval = 2 min Inflow hyds. = 10, 12, 21 Peak discharge = 48.52 cfs Time to peak = 12.73 hrs Hyd. volume = 449,716 cuft Contrib. drain. area = 4.600 ac



Hydraflow Hydrographs by Intelisolve v9.02

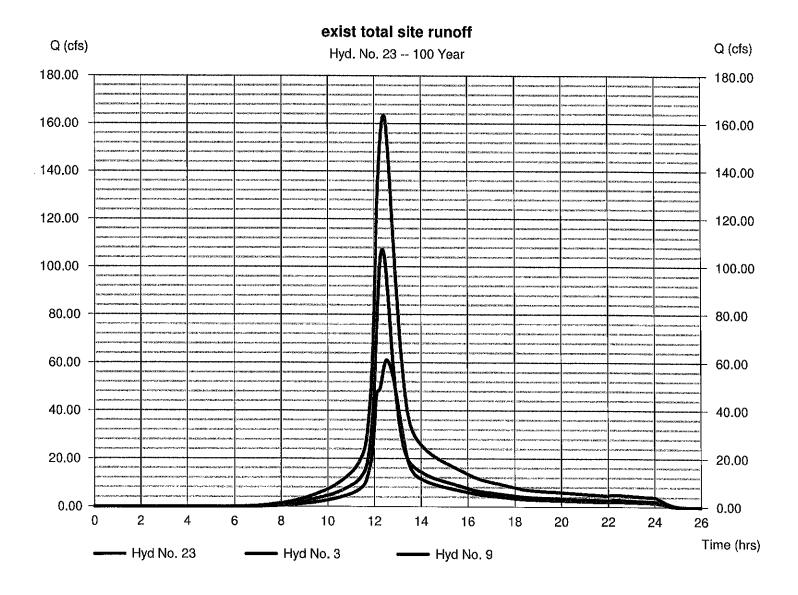
Tuesday, Aug 22, 2023

### Hyd. No. 23

exist total site runoff

Hydrograph type = Combine Storm frequency = 100 yrs Time interval = 2 min Inflow hyds. = 3, 9 Peak discharge = 163.56 cfs Time to peak = 12.43 hrs Hyd. volume = 1,136,432 cuft

Contrib. drain. area = 0.000 ac



Hydraflow Hydrographs by Intelisoive v9.02

Tuesday, Aug 22, 2023

### Hyd. No. 24

#### **DETENTION BASIN 2**

Hydrograph type = Reservoir Storm frequency = 100 yrs Time interval = 2 min

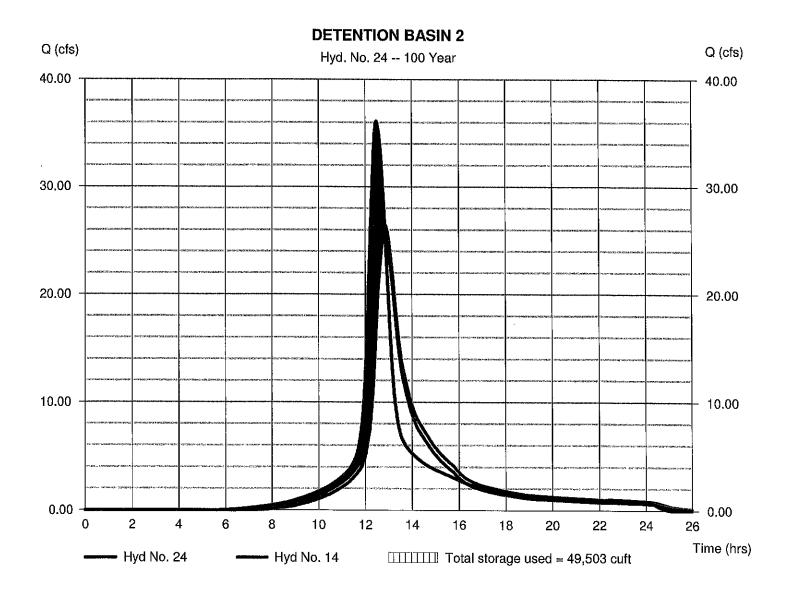
Inflow hyd. No. = 14 - PR-DA-2A-1

Reservoir name = POND 2

Peak discharge = 25.37 cfs Time to peak = 12.87 hrs

Hyd. volume = 210,043 cuft
Max. Elevation = 424.46 ft
Max. Storage = 49,503 cuft

Storage Indication method used. Exfiltration extracted from Outflow.



Tuesday, Aug 22, 2023

Pond No. 2 - POND 2

**Pond Data** 

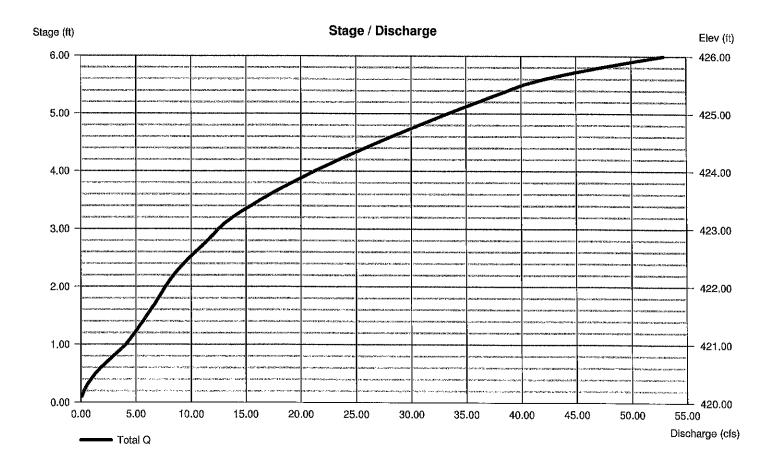
Contours - User-defined contour areas. Conic method used for volume calculation, Begining Elevation = 420,00 ft

#### Stage / Storage Table

Stage (ft)	Stage (ft) Elevation (ft) Contour area (sqft)		Incr. Storage (cuft)	Total storage (cuft)		
0.00	420.00	5,382	0	0		
1.00	421.00	8,271	6,774	6.774		
2.00	422.00	11,160	9,679	16,453		
3.00	423.00	12,968	12,051	28,504		
4.00	424.00	14,776	13,861	42,365		
5.00	425.00	16,730	15,741	58,106		
6.00	426.00	18,683	17,696	75,802		

#### **Culvert / Orifice Structures Weir Structures** [A] [B] [C] [PrfRsr] [A] [B] [C] [D] Rise (in) = 24.00 12.00 12.00 0.00 = 1.500.00 Crest Len (ft) 10.00 0.00 Span (in) = 24.0012.00 12.00 0.00 Crest El. (ft) = 423.00425.50 0.00 0.00 No. Barrels **≖** 1 2 1 0 Weir Coeff. = 3.332.60 3.33 3.33 Invert El. (ft) = 420.00420.00 422.00 0.00 Weir Type = Rect Broad Length (ft) = 60.000.00 0.00 0.00 Multi-Stage = No Yes No No Slope (%) = 2.000.00 0.00 n/a N-Value = .013 .013 .013 n/a Orifice Coeff. = 0.600,60 0,60 0.60 Exfil.(in/hr) = 3.000 (by Contour) Multi-Stage Yes = n/a Yes No TW Elev. (ft) 0.00

Note: Culvert/Orifice outflows are analyzed under inlet and outlet control. Weir risers are checked for orifice conditions.



Hydraflow Hydrographs by Intelisolve v9.02

Tuesday, Aug 22, 2023

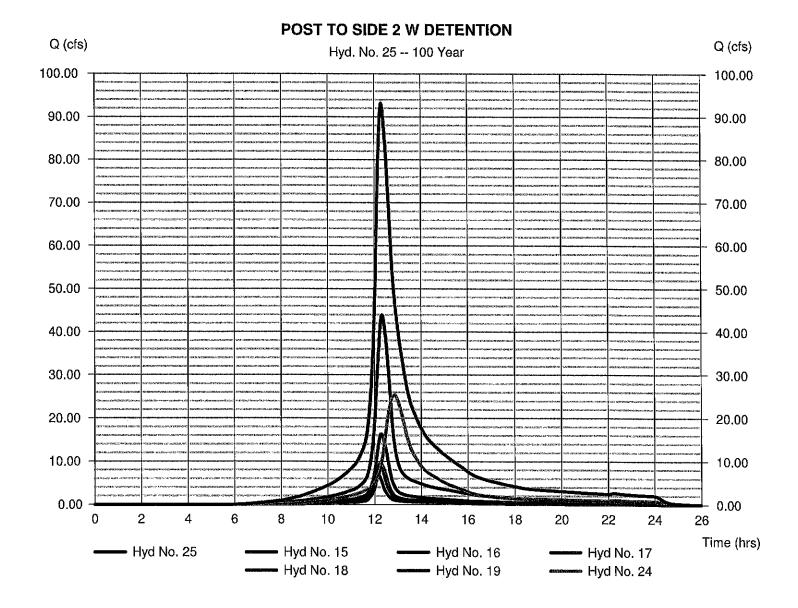
### Hyd. No. 25

#### POST TO SIDE 2 W DETENTION

Hydrograph type = Combine Storm frequency = 100 yrs Time interval = 2 min

Inflow hyds. = 15, 16, 17, 18, 19, 24

Peak discharge = 93.18 cfs Time to peak = 12.30 hrs Hyd. volume = 637,586 cuft Contrib. drain. area = 21.300 ac



Hydraflow Hydrographs by Intelisolve v9.02

Tuesday, Aug 22, 2023

### Hyd. No. 26

### POST TOTAL SITE RUNOFF

Hydrograph type = Combine Storm frequency = 100 yrs Time interval = 2 min Inflow hyds. = 22, 25

Peak discharge = 139.39 cfs Time to peak = 12.33 hrs Hyd. volume = 1,087,302 cuft

Contrib. drain. area = 0.000 ac

